JOHN WATLER BURKE

Caltragh LRD at Newtownholmes Road, Caltragh & Cornageeha, Co. Sligo

Civils Design Report

6736-JOD-XX-RP-C-0001 P.02

April 2024



Jennings O'Donovan & Partners Limited,

Consulting Engineers, Finisklin Business Park, Sligo. Tel.: 071 9161416 Fax: 071 9161080 email: info@jodireland.com



JENNINGS O'DONOVAN & PARTNERS LIMITED

Project, Civil and Structural Consulting Engineers, FINISKLIN BUSINESS PARK, SLIGO, IRELAND.

Telephone (071) 9161416 (071) 9161080 Fax

Email info@jodireland.com Web Site www.jodireland.com

DOCUMENT APPROVAL

PROJECT	Caltragh LRD	
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Prepared by

Reviewed/Approved by

Document DRAFT	Name Patrick Carr	_{Name} Eamon Morrissey
Date	Signature	Signature
March 2024	PC	Earon Morrisy

Prepared by

Reviewed/Approved by

Document FINAL	Name Patrick Carr	Name Eamon Morrissey
Date	Signature	Signature
April 2024	PC	Earon Morrisy

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Regional Director: A. Phelan Consultants: C. Birney, R. Gillan

R. Davis, S. Gilmartin, J. Healy Associates: J. McElvaney, T. McGloin, S. Molloy Associates: B. Coyle, D. Guilfoyle, L. McCormack C. O'Reilly, M. Sullivan

Company Reg No. 149104 VAT Reg. No. IE6546504D





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1 INTRODUCTION

This report has been prepared to detail the Civil Works element of the Caltragh LRD at Newtownholmes Rd., Co. Sligo. It should be read in conjunction with the foul and storm design drawings (refer to **Appendix A** for drawings) as outlined and noted herein.

This report details the foul and storm drainage design and watermain details for said development. The proposed development will entail the construction of a residential development consisting of semi-detached dwellings, detached dwellings, terraced dwellings, apartment dwellings and creche.

The proposed site, which consists of approximately 3.796 hectares, is a greenfield site. The site is located in Caltragh Co. Sligo, west of Newtownholmes Rd. It is proposed to access the site directly by vehicle via two entrances on the Newtownholmes Rd, at the eastern boundary of the site. There will be pedestrian permeability on the eastern boundary to the Newtownholmes Rd via a footpath / cycleway that will be in line with an Active Travel route.

It is proposed to direct the foul water from the development to the centre of the site where there is an existing 300mm diameter foul water pipe that crosses the site from east to west. The proposed foul water will discharge under gravity to the existing foul water network.

It is proposed to discharge the storm water generated from the development to a StormTech attenuation / infiltration storage system in the central green area of the development. Storm water generated from the development will discharge under gravity, passing through a petrol interceptor before entering the attenuation / infiltration storage system.

An Uisce Éireann Pre-connection enquiry form was submitted to Uisce Éireann in relation to the proposed development for the required wastewater and water connections. The Confirmation of Feasibility was received showing that both water and wastewater connections are feasible without upgrades from Uisce Éireann.

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2 FOUL WATER DRAINAGE DESIGN

2.1 Introduction

The drainage systems including all pipe sizes and gradients have been designed using Flow Drainage Design Software. The details of the Flow Outputs for the pipe designs are outlined in **Appendix B** of this report. The pipework to the drainage system has been designed to provide for six times the dry weather flow (DWF) in accordance with the recommendations of the Greater Dublin Strategic Drainage Study (GDSDS). It is proposed that all foul water pipes will be in accordance with Section 3.13 of the Uisce Éireann Code of Practice. The maximum pipe diameter of proposed foul water pipes is to be 225mm, with a maximum and minimum gradient such that all velocities fall within the limits of 0.75 and 2.5m/sec as set out in the "Code of Practice for Wastewater Infrastructure" by Uisce Éireann.

The foul water drainage for the development will be collected throughout the site in the foul pipe network and then directed to the centre of the site where there is an existing 300mm diameter foul water pipe that crosses the site from east to west. The proposed foul water will discharge under gravity. All of the pipe sizes and gradients are clearly indicated on the associated drawings. The typical specification for the proposed pipes is detailed in **Appendix B** and **C**. Details of the development's foul drainage network are shown on Drawing No. 6736-JOD-XX-ZZ-DR-C-200-001.

2.2 Occupancy Figures & Wastewater Flow Rates

The wastewater flow rate for the proposed development is calculated as follows in accordance with the recommendations obtained from Appendix C of the Uisce Éireann Code of Practice for Wastewater Infrastructure. Therefore, a wastewater flow rate of 150 litres/person/day was assumed. For the proposed creche, a wastewater flow rate of 180 litres/occupant/day was assumed. A detailed breakdown of the hydraulic loadings is outlined in Table 1.

Source		Hvdrauli	ic Loading (Litr	·es/Dav)
Description	Total Units	Occupancy Per Unit	Total (Litres/Day)	Total 6 DWF (Litres/Day)
A1AA – GF Apartment (Two Storey Block)	4	2	1,200	7,200
A1BA – FF Apartment (Two Storey Block)	4	2	1,200	7,200
A2AA – GF Apartment (Two Storey Block)	2	3	900	5,400
A2AB – GF Apartment (Two Storey Block)	8	3	3,600	21,600
A2BA – FF Apartment (Two Storey Block)	2	3	900	5,400
A2BB – FF Apartment (Two Storey Block)	8	3	3,600	21,600
H2CA – Two Bedroom Two Storey	8	4	4,800	28,800
H3AA – Three Bedroom Two Storey	27	5	20,250	121,500
H3AB – Three Bedroom Two Storey	9	5	6,750	40,500
H3CA – Three Bedroom Two Storey	2	4	1,200	7,200
H3CB – Three Bedroom Two Storey	2	4	1,200	7,200
H4AA – Four Bedroom Three Storey	25	7	26,250	157,500
H4AB – Four Bedroom Three Storey	8	7	8,400	50,400
H4BA – Four Bedroom Two Storey	3	6	2,700	16,200
H4BB – Four Bedroom Two Storey	5	6	4,500	27,000
H5AA – Five Bedroom Three Storey	1	9	1,350	8,100
Creche	1	32	5,760	34,560
Total	118 (+1)		94,560	567,360

Table 1 – Summary of Hydraulic Loadings for proposed Dwellings and Creche

The total hydraulic load for the proposed development is 94,560 litres per day and the proposed PE is 624. The proposed development will create an additional average daily amount of 1.095 litres / second on the existing public foul system. As outlined in Section 2.1, the proposed foul network was sized to accommodate 6 times the dry weather flow, 6.567 litres / second.

3 STORM WATER DRAINAGE DESIGN

3.1 Introduction

The storm water drainage system has been designed to cater for the proposed dwelling units' areas of hardstanding (including half-roofs, footways, roadways and car parking). The proposed storm network will collect storm water run-off throughout the development and direct it to the central green area of the development. It is proposed that all storm water generated by the site will discharge by gravity, passing through a Class 1 Klargester NSBE040 Bypass Separator or equal and similar approved, to an appropriately sized StormTech attenuation / infiltration storage system, or equal and similar approved, located within the open space in the central area of the development as shown on Drawing No. 6736-JOD-XX-ZZ-DR-C-200-001. The storm water will then infiltrate from the attenuation / infiltration storage system into the surrounding soil.

The storm drainage for the entire development has been designed in accordance with the Greater Dublin Strategic Drainage Study (GDSDS). The details of the pipe designs are outlined in **Appendix B** of this report. The storm water drainage design has been designed to cater for surface water from hard surfaces in the proposed development including roadways, footpaths, and the proposed buildings of the proposed development.

The following parameters form the basis of the design:

The surface water run-off is calculated using the Modified Rational Method (Wallingford Procedure)

$Q = 2.78 \times CV \times Cr \times I \times A$
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Where,	Q	=	rate of run-off, I/s
	Cv	=	Volumetric Run-off Coefficient
	Cr	=	Routing Coefficient
	I	=	Intensity of Rainfall, mm/hr
	А	=	Impermeable Area, hectares

- A design return period of 100 years has been adopted for the sewer network in accordance with good design practice.
- > The rainfall intensity is based on rainfall data for Caltragh.
- > Minimum self-cleansing velocity of 0.75 m/s.
- > Q-bar discharge rate shall be equal to the existing run-off rate.
- > The Principles of SuDS to be adopted for the surface water drainage.

The following impermeability factors were adopted in accordance with good design practice:

۶	Macadam Roadways	=	0.45
۶	Roof Areas	=	0.85
\triangleright	Concrete Areas	=	0.85

3.1.1 Site Drainage

Storm water run-off from the hardstanding, parking bays and footpaths will be collected by precast concrete gullies including lockable cast iron grating and frames connected to a piped system. Surface water run-off from roof areas will be collected via downpipe connections to the main network.

Gullies are located as shown on Drawing No. 6736-JOD-XX-ZZ-DR-C-200-001. Gullies are positioned in accordance with the 'Recommendations for Site Development Works.' Gullies are provided at a minimum rate of one gully per 200m².

The total storm water run-off calculated is based on the impermeable area of the site that is to be directed towards the attenuation / infiltration storage system of $17,430 \text{ m}^2$.

3.2 Proposed Attenuation

It is proposed to install an attenuation / infiltration storage system after storm manhole S02 as per Drawing No. 6736-JOD-XX-ZZ-DR-C-200-001. The proposed Stormtech attenuation / infiltration storage system is of an area approximately 1200m². Calculations for the sizing of the Stormtech attenuation / infiltration storage system are shown in **Appendix E**. Alternative products that are of equal and similar specifications can be submitted for approval prior to construction commencing.

The exact size and dimensions of the attenuation / infiltration storage system have been chosen to limit the discharge rate to an acceptable level and minimise the risk of flooding for all modelled flood events. Trial pits were excavated and tested for filtration rate. The filtration rate of the soil was estimated to be 0.03370 m/hr. At this filtrations rate, the 1 year storm discharge rate from the infiltration system is estimated to be 5.3 l/s. The 100 year storm discharge rate from the infiltration system is estimated to be 5.9 l/s. The Stormtech attenuation / infiltration storage system has a time to half empty of 1419 minutes.

A class 1 bypass petrol interceptor capable of a flowrate of 40 l/s and a drainage area greater than 20,980m² is required to be installed upstream of the attenuation tank as per Drawing No. 6736-JOD-XX-ZZ-DR-C-200-001. A Klargester Bypass Separator NSBE040 or equal and similar approved is proposed.

3.3 SuDS Principles

The key SuDS principles that influence the planning and design process, enabling SuDS to mimic natural drainage are:

- Storing runoff and releasing it slowly (attenuation)
- Harvesting and using the rain close to where it falls.
- Allowing water to soak into the ground (infiltration)
- Slowly transporting (conveying) water on the surface.
- Filtering out pollutants.
- Allowing sediments to settle out by controlling the flow of the water.

The proposed drainage scheme takes into account a number of the above listed principles through the following measures:

- The proposed attenuation / infiltration storage system stores runoff and releases it slowly into the surrounding ground via infiltration.
- Proposing water butts and local soakaways to the rear of dwellings to allow for re use and infiltration of roof water for each dwelling.
- Providing public open space green areas allowing rainfall to naturally percolate into the ground.
- Strategic placing of gullies to keep road surface gradients as gentle as possible to cater for the slow transporting of water on the surface.
- Proposing a class 1 petrol/oil interceptor to remove pollutants from the system.

4 WATER MAIN

The water main has been designed in accordance with the Uisce Éireann Code of Practice for Water Infrastructure. A 180mm OD PE connection is proposed to be made to the existing 150mm inside diameter uPVC watermain which is laid across the central area of the proposed site, as shown on Drawing No. 6736-JOD-XX-ZZ-DR-C-200-007, included in **Appendix A**. From the proposed 180mm OD PE watermain, it is proposed to connect 110mm OD PE watermains to serve the branches of the proposed development. A 25mm PE connection will be made to each dwelling/unit.

Hydrants will be positioned within the site such that:

- The distance from each building is not less than 6m or more than 46m,
- The distance from a hydrant to a vehicle access road or hard-standing area for fire appliances is not more than 30m,
- They are distributed around the perimeter of the buildings, having regard for the provision of access for fire appliances, (as per Building Regulations 2006 Technical Guidance Document B)

The hydrants shall be capable of delivering a minimum of 8 litres per second through any single hydrant as per Water UK – National Guidance Document on the Provision of Water for Fire Fighting.

In accordance with Uisce Éireann standards a bulk flow meter and sluice valves are proposed at the connection point of the existing watermain, adjacent to the north eastern entrance of the proposed development. All water mains are to be commissioned and pressure tested to Uisce Éireann Standards. The typical connection details and meter details are shown in Uisce Éireann Water Standard Construction Details IW-CDS-5020-01.

5 CONCLUSION

The Report should be read in conjunction with the associated Drawings, layouts and specifications. The wastewater drainage layout and storm water drainage layout meet the requirements of the proposed development and are in accordance with the relevant codes of practice and standards.

APPENDIX A

DRAWINGS



EWATER PIPE WORK SHALL BE INSTALLED IN ANCE WITH I.W. STD-WW-05, STD-WW-06, AND D6A REGARDING SEPARATION ES.WHERE SEPARATION DISTANCES CANNOT VED, PIPES SHALL BE SURROUND IN LEAN MIX TE.	 GENERAL NOTES: 1 FIGURED DIMENSIONS ONLY TO BE TAKEN FROM THIS DRAWING. 2 ALL DRAWINGS TO BE CHECKED BY THE CONTRACTOR ON SITE. 3 ENGINEER TO BE INFORMED OF ANY DISCREPANCIES BEFORE ANY WORK COMMENCES. 4 THIS DRAWING TO BE READ IN CONJUNCTION WITH ALL OTHER RELEVANT DRAWINGS AND SPECIFICATIONS.
TONS FROM FOUL AND SURFACE WATER	
ANCE WITH I.W. STD-WW-03-1 IN COMPLIANCE H WATER CODE OF PRACTICE AND STANDARD	SITE BOUNDARY shown thus
VER DROP MANHOLES TO BE INSTALLED TO IW	PROPOSED 1200MM D400 STORM/FOUL
ATER CODE OF PRACTICE STANDARD DETAIL 12.	
NWALL PIPES TO BE USED FOR ALL STORM PES UNLESS OTHERWISE SPECIFIED. ALL	D400 STORM/FOUL MANHOLE shown thus
VER PIPES TO BE IN ACCORDANCE WITH IRISH ODES OF PRACTICE.	LOW COVER STORM/FOUL PIPES shown thus
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	PROPOSED FOUL NETWORK shown thus
D, PIPES SHALL BE SURROUNDED IN LEAN MIX E.	CONNECTIONS shown thus
	PROPOSED FOUL WATER CONNECTIONS
	PROPOSED STORM/FOUL WATER AJs
	BOUNDARY BOX TO IW-STD-WW-13 shown thus
	PROPOSED ROAD GULLIES shown thus
	EXISTING FOUL WATER PIPE shown thus
	Red Line Area:- 37,960 m ² 3.796 Hectares
	ITM Co-Ordinates of site:- Irish Grid Co-Ordinates: 568884, 834363 168926, 334357
	Ordnance Survey Ireland Licence No.CYAL50382164 © Ordnance Survey of Ireland/Government of Ireland.
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	P.04 Revised Site Layout PC EM EM 29.04.24 P.03 Revised Site Layout KK EM EM 24.04.24
	P.02 Issued for PLANNING PC EM EM 22.03.23 P.01 Issued for LRD PC EM EM 05.10.23
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© COPYRIGH JENNIN CONSUL FINISKLI SLIGO, IRELAND TEL. +353 (0) FAX. +353 (0) Email: info@	IT OF IGS O'DON TING ENGIN N,)71 916 1416)71 916 1080 jodireland.com	OVAN & I EERS,			NERS
drawing no. 6736-JOE	D-XX-ZZ-DR-	C-200-002		re	vision P.02



















NOTES GENERAL NOTES: FIGURED DIMENSIONS ONLY TO BE TAKEN FROM THIS DRAWING. ALL DRAWINGS TO BE CHECKED BY THE CONTRACTOR ON SITE ENGINEER/EMPLOYERS REPRESENTATIVE, AS APPROPRIATE, TO BE INFORMED BY THE CONTRACTOR OF ANY DISCREPANCIES BEFORE ANY WORK COMMENCES THE CONTRACTOR SHALL UNDERTAKE A THOROUGH CHECK FOR THE ACTUAL LOCATION OF ALL SERVICES/UTILITIES, ABOVE AND BELOW GROUND, BEFORE ANY WORK COMMENCES ALL LEVELS SHOWN RELATE TO ORDNANCE SURVEY DATUM AT MALIN HEAD THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL OTHER RELEVANT DRAWINGS AND SPECIFICATIONS. CONTRACTOR TO VERIFY THE ACCURACY OF THIS PROPOSAL TO THE ENGINEER AND ALLOW FOR MINOR CORRECTIONS AS DEEMED NECESSARY WITH A REASONABLE TIMEFRAME. BITUMINOUS CONSTRUCTION WITH COLOURED FINISH TO ARCHITECTS DETAIL 1000 TABLETOP 70 800 000 TRANSITION RAMP 4 44 4 *{///¥{/{/*{/////~ P.03 Added transition ramp detail PC EM EM 29.04.24 00 P.02 Issued for PLANNING PC EM EM 22.03.24 500 PC EM EM 05.10.23 P.01 Issued for LRD Rev. Modifications By Chkd Aprvd Date Layout Ref.: P:\Jod-jobs\6736 Burkes Cornageeha\700 Drawings\703 File Planning\01 WIP\6736-JOD-XX-ZZ-DR-C-200-008-Road Lavout dwo Client JOHN WALTER BURKE Project CALTRAGH LRD Stage PLANNING Title GULLY COVER SLAB PROPOSED ROAD SECTION A-A AND ROAD CONSTRUCTION DETAILS - 5 TO 10 RECESS, GULLY PROFILE TO FOLLOW ROAD PROFILE Scale MORTAR BED (10 MIN 20 MAX) As Shown @ A1 AND HAUNCH TO FRAME TO CLAUSE 507.13 Checked Date Surveyed Drawn CLASS A ENGINEERING BRICKWORK - TO IS 91 AND TO 2400 SERIES OCT '23 PC EM OR IN-SITU CONCRETE RISER GULLY COVER SLAB (MIN THICKNESS 100) WHERE REQUIRED © COPYRIGHT OF └ MORTAR TO 2400 SERIES JENNINGS O'DONOVAN & PARTNERS CONSULTING ENGINEERS,

- MIX ST4 CONCRETE OR TO MANUFACTURERS REQUIREMENTS

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GENERAL NOTES: FIGURED DIMENSIONS ONLY TO BE TAKEN FROM THIS DRAWING. 2 ALL DRAWINGS TO BE CHECKED BY THE CONTRACTOR ON SITE. 3 ENGINEER TO BE INFORMED OF ANY DISCREPANCIES BEFORE ANY WORK COMMENCES. 4 THIS DRAWING TO BE READ IN CONJUNCTION WITH ALL OTHER RELEVANT DRAWINGS AND SPECIFICATIONS. LEGEND Phoenix 2 Duo (P2-12W with Elite 6x4 chassis) Overall Length 10.200m Overall Width 2.530m Overall Body Height 3.751m Min Body Ground Clearance 0.304m Track Width 2.500m Lock-to-lock time 4.00s Curb to Curb Turning Radius 7.800m CHASSIS PATH shown thus BODY PATH shown thus Red Line Area:-37,960 m² 3.796 Hectares ITM Co-Ordinates of site:-Irish Grid Co-Ordinates: 568884, 834363 168926, 334357 PC EM EM 29.04.24 P.04 Amended site layout P.03 Issued for PLANNING PC EM EM 22.03.24 P.02 Amended creche road to one way PC EM EM 10.10.23 P.01 Issued for LRD PC EM EM 05.10.23 By Chkd Aprvd Date Rev. Modifications Layout Ref.: P:\Jod-jobs\6736 Burkes Cornageeha\700 Drawings\703 File Planning\01 WIP\6736-JOD-XX-ZZ-DR-C-200-011-012-Autotrack Analysis dwg Client JOHN WALTER BURKE Project CALTRAGH LRD Stage PLANNING Title PROPOSED ROAD LAYOUT - SWEPT PATH ANALYSIS SHEET 1 OF 2 Scale AS SHOWN @ A1 Checked Date Drawn Surveyed PC EM OCT '23 © COPYRIGHT OF JENNINGS O'DONOVAN & PARTNERS CONSULTING ENGINEERS, FINISKLIN, SLIGO, IRELAND. TEL. +353 (0)71 916 1416 FAX. +353 (0)71 916 1080 Email: info@jodireland.com

NOTES

6736-JOD-XX-ZZ-DR-C-200-011

Drawing No.

Revision P.04











NOTES				
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LEGEND				
Phoenix 2 Duo (P2-12W with Uverall Length Uverall Width Uverall Body Height Min Body Ground Clearance Track Width Lock-to-lock time Curb to Curb Turning Radius	Elite 6x4 chassis) 10.200m 2.530m 3.751m 0.304m 2.500m 4.00s 7.800m			
CHASSIS PATH shown thus				
BODY PATH shown thus				
Red Line Area:- 37,960 m ² 3.796 Hectares ITM Co-Ordinates of site:- 568884, 834363 Irish Grid Co-Ordinates: 168926, 334357				
P.04 Amended site layout	PC EM EM 29.04.24			
P.03 Issued for PLANNING P.02 Amended creche road to one way	PC EM EM 22.03.24 PC EM EM 10.10.23			
P.01 Issued for LRD Rev. Modifications	PC EM EM 05.10.23 By Chkd Apryd Date			
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Client	5			
JOHN WALTER BURKE				
Project CALTRAGH LRD				
Stage				
PLANNING Title				
PROPOSED ROAD LAYOUT - SWEPT				
PATH ANALYSIS SHEET 2	OF 2			
Scale				
AS SHOWN @ A1	ed Data			
PC EM	OCT '23			
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Drawing No.	Dovision			
6736-JOD-XX-ZZ-DR-C-200-	012 P.04			

APPENDIX B

DESIGN CALCULATIONS

Design Settings

Frequency of use (kDU)	0.50	Minimum Velocity (m/s)	0.75
Flow per dwelling per day (I/day)	450	Connection Type	Level Inverts
Domestic Flow (I/s/ha)	0.0	Minimum Backdrop Height (m)	0.600
Industrial Flow (I/s/ha)	0.0	Preferred Cover Depth (m)	1.200
Additional Flow (%)	0	Include Intermediate Ground	\checkmark

Adoptable Manhole Type

Max Width (mm)	Diameter (mm)	Max Width (mm)	Diameter (mm)
374	1200	749	1500
499	1350	900	1800

>900 Link+900 mm

Max Depth (m)	Diameter (mm)	Max Depth (m)	Diameter (mm)
1.500	1050	99.999	1200

Circular Link Type

Shape	Circular	Auto Increment (mm)	75
Barrels	1	Follow Ground	х

Available Diameters (mm)

100 150

<u>Nodes</u>

Name	Dwellings	Cover Level (m)	Manhole Type	Easting (m)	Northing (m)	Depth (m)
9105		50.502	Adoptable	568901.588	834150.658	3.540
9208		50.731	Adoptable	568916.525	834211.240	4.040
9202		49.052	Adoptable	568900.367	834221.592	3.485
9201		46.310	Adoptable	568911.763	834258.397	6.212
9306		42.730	Adoptable	568930.626	834321.739	2.805
9302		39.070	Adoptable	568879.840	834365.334	5.144
F07-2	4	46.750	Adoptable	568867.260	834225.090	0.790
F07-1	6	46.380	Adoptable	568895.148	834232.591	0.900
F07	8	46.410	Adoptable	568904.424	834229.622	1.090
F06-2	16	41.870	Adoptable	568847.420	834280.612	1.470
F06-1	17	43.550	Adoptable	568904.886	834294.610	3.550
F06	25	44.285	Adoptable	568923.205	834291.218	4.875
F05	26	42.750	Adoptable	568932.622	834321.601	3.505
F04-3	12	45.540	Adoptable	568912.000	834465.350	1.890
F04-2	20	41.715	Adoptable	568911.621	834414.502	1.430
F04-1	22	40.680	Adoptable	568909.517	834358.247	1.425
F04	48	40.470	Adoptable	568901.481	834348.597	1.515
F03-2	8	37.380	Adoptable	568831.326	834345.901	0.750
F03-1	8	39.180	Adoptable	568879.733	834357.746	3.135
F03	59	39.300	Adoptable	568884.537	834363.287	3.380
F02-5	10	43.540	Adoptable	568802.513	834541.774	1.390
F02-4	10	45.210	Adoptable	568855.221	834518.411	3.460
F02-3-1	7	40.440	Adoptable	568793.259	834509.218	1.740
F02-3	22	44.750	Adoptable	568852.777	834509.693	7.040
F02-2	33	41.410	Adoptable	568841.978	834468.813	3.910

<u>Nodes</u>

N	ame	Dwellings	Cover Level (m)	Manhole Type	Eastir (m)	ng	Northing (m)	Dept (m)	h
FC	02-1	36	37.870	Adoptable	568842	.524	834422.455	5 1.71	.5
FC	02	95	36.470	Adoptable	568836	.581	834404.886	5 1.29	5
FC	01-2	3	36.145	Adoptable	568801	.338	834447.071	L 1.44	5
FC	01-1	2	36.130	Adoptable	568804	.667	834431.126	5 1.62	0
FC	F01 98		36.315	Adoptable	568823	.495	834416.093	3 2.15	5
84	8403		36.280	Adoptable	568822	.647	834415.009	9 3.31	.3
84	401		34.374	Adoptable	568750	.538	834472.522	L 3.20	0
				<u>Links</u>					
Name	US	DS	Length	ks (mm) /	US IL	DS I	L Fall	Slope	Dia
	Noc	le Node	(m)	n	(m)	(m)	(m)	(1:X)	(mm)
FP9105	5 910	5 9208	62.396	1.500	46.962	46.69	0.271	230.2	300
FP9208	920	8 9202	19.190	1.500	46.691	45.56	57 1.124	17.1	300
FP9202	920	2 9201	38.529	1.500	45.567	40.09	5.469	7.0	300
FP9201	L 920	1 9306	66.091	1.500	40.098	39.92	.5 0.173	382.0	300
FP9306	5 930	6 9302	66.931	1.500	39.925	33.92	6 5.999	11.2	300
FP9302	2 930	2 8403	75.754	1.500	33.926	32.96	0.959	79.0	300
FP07-2	F07	-2 F07-1	28.879	1.500	45.960	45.48	30 0.480	60.2	150
FP07-1	F07	-1 F07	9.740	1.500	45.480	45.32	0 0.160	60.9	150
FP07	F07	F06	64.396	1.500	45.320	42.62	20 2.700	23.9	150
FP06-2	F06	-2 F06-1	59.146	1.500	40.400	40.00	0 0.400	147.9	150
FP06-1	F06	-1 F06	18.630	1.500	40.000	39.41	.0 0.590	31.6	150
FP06	F06	F05	31.809	1.500	39.410	39.24	5 0.165	192.8	225
FP05	F05	F04	41.213	1.500	39.245	38.95	5 0.290	142.1	225
FP04-3	F04	-3 F04-2	50.849	1.500	43.650	40.28	3.365	15.1	150
FP04-2	F04	-2 F04-1	52.015	1.500	40.285	39.25	5 1.030	50.5	225
FP04-1	F04	-1 F04	16.241	1.500	39.255	38.95	5 0.300	54.1	225
FP04	F04	F03	22.425	1.500	38.955	37.41	.5 1.540	14.6	225
FP03-2	F03	-2 F03-1	49.835	1.500	36.630	36.04	5 0.585	85.2	150

Name	Pro Vel	Vel	Сар	Flow	US	DS	Σ Area	Σ Dwellings	Σ Units	Σ Add	Pro	Pro
	@ 1/3 Q	(m/s)	(I/s)	(I/s)	Depth	Depth	(ha)	(ha)	(ha)	Inflow	Depth	Velocity
	(m/s)				(m)	(m)				(ha)	(mm)	(m/s)
FP9105	0.000	0.912	64.5	0.0	3.240	3.740	0.000	0	0.0	0.0	0	0.000
FP9208	0.000	3.363	237.7	0.0	3.740	3.185	0.000	0	0.0	0.0	0	0.000
FP9202	0.000	5.239	370.3	0.0	3.185	5.912	0.000	0	0.0	0.0	0	0.000
FP9201	0.000	0.707	50.0	0.0	5.912	2.505	0.000	0	0.0	0.0	0	0.000
FP9306	0.000	4.162	294.2	0.0	2.505	4.844	0.000	0	0.0	0.0	0	0.000
FP9302	0.000	1.561	110.3	0.0	4.844	3.013	0.000	0	0.0	0.0	0	0.000
FP07-2	0.113	1.130	20.0	0.0	0.640	0.750	0.000	4	0.0	0.0	4	0.154
FP07-1	0.153	1.124	19.9	0.1	0.750	0.940	0.000	10	0.0	0.0	6	0.225
FP07	0.248	1.799	31.8	0.1	0.940	1.515	0.000	18	0.0	0.0	6	0.363
FP06-2	0.132	0.719	12.7	0.1	1.320	3.400	0.000	16	0.0	0.0	9	0.192
FP06-1	0.291	1.562	27.6	0.2	3.400	4.725	0.000	33	0.0	0.0	9	0.421
FP06	0.194	0.825	32.8	0.4	4.650	3.280	0.000	76	0.0	0.0	18	0.277
FP05	0.227	0.962	38.2	0.5	3.280	1.290	0.000	102	0.0	0.0	19	0.334
FP04-3	0.273	2.261	40.0	0.1	1.740	1.280	0.000	12	0.0	0.0	5	0.388
FP04-2	0.235	1.617	64.3	0.2	1.205	1.200	0.000	32	0.0	0.0	8	0.314
FP04-1	0.253	1.562	62.1	0.3	1.200	1.290	0.000	54	0.0	0.0	11	0.371
FP04	0.635	3.016	119.9	1.1	1.290	1.660	0.000	204	0.0	0.0	15	0.915
FP03-2	0.129	0.949	16.8	0.0	0.600	2.985	0.000	8	0.0	0.0	6	0.175

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	Ltd.													
	Finisklin Busi	ness Par	'k		Patrio	ck Carr								
	Co. Sligo				21/03	3/2024								
						Links								
	Name	e	US	DS	Length	ks (mm)	/ US II	L DS IL	Fall	Slope	Dia			
		N	ode	Node	(m)	n	(m)	(m)	(m)	(1:X)	(mm)			
	FP03-1	FO	3-1	F03	7.334	1.50	36.04	5 35.920	0.125	58.7	150			
	FP03	FO	3	F02	63.484	1.50	35.92	0 35.175	0.745	85.2	225			
	FP02-5	FO	2-5	F02-4	57.654	1.50	42.15	0 41.750	0.400	144.1	150			
	FP02-4	F0	2-4	F02-3	9.054	1.50	41.75	0 41.690	0.060	150.9	150			
	FP02-3-	-1 F0	2-3-1	F02-3	59.520	1.50	38.70	0 37.710	0.990	60.1	150			
	FP02-3	FO:	2-3	F02-2	42.282	1.50	37.71	.0 37.500	0.210	201.3	225			
	FP02-2	F0	2-2	F02-1	46.361	1.50	37.50	0 36.155	1.345	34.5	225			
	FP02-1	F0	2-1	F02	18.547	1.50	36.15	5 35.175	0.980	18.9	225			
	FP02	F0	2	F01	20.161	1.50	35.17	5 34.760	0.415	48.6	225			
	FP01-2 F01-2 F01-1				11.459	1.50	34.70	0 34.510	0.190	60.3	150			
	FP01-1 F01-1 F01				21.163	1.50	34.51	0 34.160	0.350	60.5	150			
	FP01	FO	1	8403	3.294	1.50	34.16	0 34.145	0.015	219.6	225			
	8403_E	XG 84	03	8401	92.235	1.50	32.96	31.174	1.793	51.4	300			
Name	Pro Vel	Vel	Сар	Flow	US	DS	Σ Area	Σ Dwelling	s ΣUni	ts ΣAd	ld Pro	Pro		
	@ 1/3 Q	(m/s)	(I/s)	(I/s)	Depth	Depth	(ha)	(ha)	(ha)) Inflo	w Depth	Velocity		
	(m/s)				(m)	(m)				(ha) (mm)	(m/s)		
FP03-1	0.176	1.145	20.2	0.1	2.985	3.230	0.000	16	50	.0 0	.0 7	0.262		
FP03	0.390	1.244	49.5	1.5	3.155	1.070	0.000	279	90	.0 0	.0 27	0.548		
FP02-5	0.110	0.729	12.9	0.1	1.240	3.310	0.000	10) 0	.0 0	.0 7	0.165		
FP02-4	0.141	0.712	12.6	0.1	3.310	2.910	0.000	20) 0	.0 0	.0 10	0.208		
FP02-3-1	0.134	1.131	20.0	0.0	1.590	6.890	0.000	7	7 O	.0 0	.0 5	0.192		
FP02-3	0.167	0.807	32.1	0.3	6.815	3.685	0.000	49	90	.0 0	.0 15	0.233		
FP02-2	0.351	1.959	77.9	0.4	3.685	1.490	0.000	82	2 0	.0 0	.0 12	0.493		
FP02-1	0.475	2.645	105.2	0.6	1.490	1.070	0.000	118	30	.0 0	.0 13	0.703		
FP02	0.557	1.649	65.6	2.6	1.070	1.330	0.000	492	2 0	.0 0	.0 30	0.789		
FP01-2	0.090	1.129	20.0	0.0	1.295	1.470	0.000	3	30	.0 0	.0 4	0.154		
FP01-1	0.112	1.128	19.9	0.0	1.470	2.005	0.000	5	5 0	.0 0	.0 4	0.173		
FP01	0.354	0.773	30.7	3.1	1.930	1.910	0.000	595	5 0	.0 0	.0 49	0.497		
8403_EX0	0.566	1.935	136.8	3.1	3.013	2.900	0.000	595	50	.0 0	.0 31	0.793		

Pipeline Schedule

Link	Length (m)	Slope (1:X)	Dia (mm)	Link Type	US CL (m)	US IL (m)	US Depth (m)	DS CL (m)	DS IL [(m)	OS Depth (m)
FP9105	62.396	230.2	300	Circular	50.502	46.962	3.240	50.731	46.691	3.740
FP9208	19.190	17.1	300	Circular	50.731	46.691	3.740	49.052	45.567	3.185
FP9202	38.529	7.0	300	Circular	49.052	45.567	3.185	46.310	40.098	5.912
FP9201	66.091	382.0	300	Circular	46.310	40.098	5.912	42.730	39.925	2.505
FP9306	66.931	11.2	300	Circular	42.730	39.925	2.505	39.070	33.926	4.844
FP9302	75.754	79.0	300	Circular	39.070	33.926	4.844	36.280	32.967	3.013
FP07-2	28.879	60.2	150	Circular	46.750	45.960	0.640	46.380	45.480	0.750
	Link	US	Dia	Node	МН	DS	Dia	Node	МН	
		Node	(mm)	Туре	Туре	Node	e (mm)	Туре	Туре	
	FP9105	9105	1200	Manhole	Adoptab	le 9208	1200	Manhole	Adoptable	2
	FP9208	9208	1200	Manhole	Adoptab	le 9202	1200	Manhole	Adoptable	2
	FP9202	9202	1200	Manhole	Adoptab	le 9201	1200	Manhole	Adoptable	2
	FP9201	9201	1200	Manhole	Adoptab	le 9306	1200	Manhole	Adoptable	2
	FP9306	9306	1200	Manhole	Adoptab	le 9302	1200	Manhole	Adoptable	2
	FP9302	9302	1200	Manhole	Adoptab	le 8403	1200	Manhole	Adoptable	2
	FP07-2	F07-2	1200	Manhole	Adoptab	le F07-1	L 1200	Manhole	Adoptable	2

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					Pi	peline Sch	<u>nedule</u>					
	Link	Length (m)	Slope (1:X)	Dia (mm)	Link Type	US CL (m)	US IL (m)	US Depth (m)	DS CL (m)	DS IL (m)	DS Depth (m)	
	FP07-1	9.740	60.9	150	Circular	46.380	45.480	0.750	46.410	45.320	0.940	
	FP07	64.396	23.9	150	Circular	46.410	45.320	0.940	44.285	42.620	1.515	
	FP06-2	59.146	147.9	150	Circular	41.870	40.400	1.320	43.550	40.000	3.400	
	FP06-1	18.630	31.6	150	Circular	43.550	40.000	3.400	44.285	39.410	4.725	
	FP06	31.809	192.8	225	Circular	44.285	39.410	4.650	42.750	39.245	3.280	
	FP05	41.213	142.1	225	Circular	42.750	39.245	3.280	40.470	38.955	1.290	
	FP04-3	50.849	15.1	150	Circular	45.540	43.650	1.740	41.715	40.285	1.280	
	FP04-2	52.015	50.5	225	Circular	41.715	40.285	1.205	40.680	39.255	1.200	
	FP04-1	16.241	54.1	225	Circular	40.680	39.255	1.200	40.470	38.955	1.290	
	FP04	22.425	14.6	225	Circular	40.470	38.955	1.290	39.300	37.415	1.660	
	FP03-2	49.835	85.2	150	Circular	37.380	36.630	0.600	39.180	36.045	2.985	
	FP03-1	7.334	58.7	150	Circular	39.180	36.045	2.985	39.300	35.920	3.230	
	FP03	63.484	85.2	225	Circular	39.300	35.920	3.155	36.470	35.175	1.070	
	FP02-5	57.654	144.1	150	Circular	43.540	42.150	1.240	45.210	41.750	3.310	
	FP02-4	9.054	150.9	150	Circular	45.210	41.750	3.310	44.750	41.690	2.910	
	FP02-3-1	59.520	60.1	150	Circular	40.440	38.700	1.590	44.750	37.710	6.890	
	FP02-3	42.282	201.3	225	Circular	44.750	37.710	6.815	41.410	37.500	3.685	
	FP02-2	46.361	34.5	225	Circular	41.410	37.500	3.685	37.870	36.155	1.490	
	FP02-1	18.547	18.9	225	Circular	37.870	36.155	1.490	36.470	35.175	1.070	
	FP02	20.161	48.6	225	Circular	36.470	35.175	1.070	36.315	34.760	1.330	
	FP01-2	11.459	60.3	150	Circular	36.145	34.700	1.295	36.130	34.510	1.470	
	FP01-1	21.163	60.5	150	Circular	36.130	34.510	1.470	36.315	34.160	2.005	
	FP01	3.294	219.6	225	Circular	36.315	34.160	1.930	36.280	34.145	1.910	
:	8403_EXG	92.235	51.4	300	Circular	36.280	32.967	3.013	34.374	31.174	2.900	

Link	US	Dia	Node	MH	DS	Dia	Node	MH
	Node	(mm)	Туре	Туре	Node	(mm)	Туре	Туре
FP07-1	F07-1	1200	Manhole	Adoptable	F07	1200	Manhole	Adoptable
FP07	F07	1200	Manhole	Adoptable	F06	1200	Manhole	Adoptable
FP06-2	F06-2	1200	Manhole	Adoptable	F06-1	1200	Manhole	Adoptable
FP06-1	F06-1	1200	Manhole	Adoptable	F06	1200	Manhole	Adoptable
FP06	F06	1200	Manhole	Adoptable	F05	1200	Manhole	Adoptable
FP05	F05	1200	Manhole	Adoptable	F04	1200	Manhole	Adoptable
FP04-3	F04-3	1200	Manhole	Adoptable	F04-2	1200	Manhole	Adoptable
FP04-2	F04-2	1200	Manhole	Adoptable	F04-1	1200	Manhole	Adoptable
FP04-1	F04-1	1200	Manhole	Adoptable	F04	1200	Manhole	Adoptable
FP04	F04	1200	Manhole	Adoptable	F03	1200	Manhole	Adoptable
FP03-2	F03-2	1200	Manhole	Adoptable	F03-1	1200	Manhole	Adoptable
FP03-1	F03-1	1200	Manhole	Adoptable	F03	1200	Manhole	Adoptable
FP03	F03	1200	Manhole	Adoptable	F02	1200	Manhole	Adoptable
FP02-5	F02-5	1200	Manhole	Adoptable	F02-4	1200	Manhole	Adoptable
FP02-4	F02-4	1200	Manhole	Adoptable	F02-3	1200	Manhole	Adoptable
FP02-3-1	F02-3-1	1200	Manhole	Adoptable	F02-3	1200	Manhole	Adoptable
FP02-3	F02-3	1200	Manhole	Adoptable	F02-2	1200	Manhole	Adoptable
FP02-2	F02-2	1200	Manhole	Adoptable	F02-1	1200	Manhole	Adoptable
FP02-1	F02-1	1200	Manhole	Adoptable	F02	1200	Manhole	Adoptable
FP02	F02	1200	Manhole	Adoptable	F01	1200	Manhole	Adoptable
FP01-2	F01-2	1200	Manhole	Adoptable	F01-1	1200	Manhole	Adoptable
FP01-1	F01-1	1200	Manhole	Adoptable	F01	1200	Manhole	Adoptable
FP01	F01	1200	Manhole	Adoptable	8403	1200	Manhole	Adoptable
8403_EXG	8403	1200	Manhole	Adoptable	8401	1200	Manhole	Adoptable

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Ltd.	Network: Foul Network	
Finisklin Business Park	Patrick Carr	
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Node	Easting (m)	Northing (m)	CL (m)	Depth (m)	Dia (mm)	Connection	s	Link	IL (m)	Dia (mm)
9105	568901.588	834150.658	50.502	3.540	1200	Ċ				
							0	FP9105	46.962	300
9208	568916.525	834211.240	50.731	4.040	1200	° ~	1	FP9105	46.691	300
						1′	0	FP9208	46.691	300
9202	568900.367	834221.592	49.052	3.485	1200	° (1	FP9208	45.567	300
0201	FC0011 7C2	024250 207	46 210	C 212	1200		0	FP9202	45.567	300
9201	568911.763	834258.397	46.310	6.212	1200	\oint	1	FP9202	40.098	300
						1	0	FP9201	40.098	300
9306	568930.626	834321.739	42.730	2.805	1200	° Ç	1	FP9201	39.925	300
						1′	0	FP9306	39.925	300
9302	568879.840	834365.334	39.070	5.144	1200	°r	1	FP9306	33.926	300
						1	0	FP9302	33.926	300
F07-2	568867.260	834225.090	46.750	0.790	1200	⊖→ ⁰				
507.4			46.000		4000		0	FP07-2	45.960	150
F07-1	568895.148	834232.591	46.380	0.900	1200	1	1	FP07-2	45.480	150
F07	E69004 424	924220 622	46 410	1 000	1200	0	0	FP07-1	45.480	150
FU7	508904.424	834229.022	40.410	1.090	1200	1	T	FP07-1	45.320	150
							0	FP07	45.320	150
F06-2	568847.420	834280.612	41.870	1.470	1200	⊖→0				450
506.4	560004 006	024204 640	42 550	2 5 5 0	1200		0	FP06-2	40.400	150
F06-1	568904.886	834294.610	43.550	3.550	1200	1	1	FP06-2	40.000	150
							0	FP06-1	40.000	150
F06	568923.205	834291.218	44.285	4.875	1200	Ŷ	1	FP06-1	39.410	150
							2	FP07	42.620	150 225
F05	568932 622	834321 601	42 750	3 505	1200	-	1	FP06	39 245	225
	500552.022	554521.001	72.750	5.505	1200	°	Ŧ	1100	55.245	223
						1′	0	FP05	39.245	225

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Ltd.	Network: Foul Network	
Finisklin Business Park	Patrick Carr	
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Node	Easting (m)	Northing (m)	CL (m)	Depth (m)	Dia (mm)	Connection	S	Link	IL (m)	Dia (mm)
F04-3	568912.000	834465.350	45.540	1.890	1200					
						\bigcirc				
						ő	0	FP04-3	43.650	150
F04-2	568911.621	834414.502	41.715	1.430	1200		1	FP04-3	40.285	150
						0	0	FP04-2	40.285	225
F04-1	568909.517	834358.247	40.680	1.425	1200		1	FP04-2	39.255	225
						U	0	FP04-1	39.255	225
F04	568901.481	834348.597	40.470	1.515	1200	° 5 1	1	FP04-1	38.955	225
							2	FP05	38.955	225
							0	FP04	38.955	225
F03-2	568831.326	834345.901	37.380	0.750	1200	⊖→ ⁰				
							0	FP03-2	36.630	150
F03-1	568879.733	834357.746	39.180	3.135	1200	1-07	1	FP03-2	36.045	150
							0	FP03-1	36.045	150
F03	568884.537	834363.287	39.300	3.380	1200	0	1	FP03-1	35.920	150
							2	FP04	37.415	225
E03 E	E60002 E12	924541 774	12 5 10	1 200	1200		0	FP03	35.920	225
FU2-5	506602.515	654541.774	45.540	1.590	1200	\bigcirc	-			
F02 4	FC00FF 224	024510 411	45 210	2 4 6 0	1200		0	FP02-5	42.150	150
FUZ-4	568855.221	834518.411	45.210	3.460	1200		T	FP02-5	41.750	150
502.2.4	560702.250	024500.240	10.110	4 740	4200	0	0	FP02-4	41.750	150
FU2-3-1	568793.259	834509.218	40.440	1.740	1200	⊖→0	•	5000.0.4	20 700	450
502.2		824500 602	44 750	7 0 4 0	1200		0	FP02-3-1	38.700	150
F02-3	568852.777	834509.693	44.750	7.040	1200		1	FP02-3-1	37.710	150
						1	2	FP02-4	41.690	150
E02.2	EC00/1 070	024460 012	41 410	2 010	1200	0	1	FPU2-3	37.710	225
FUZ-Z	508841.978	834408.813	41.410	3.910	1200	ϕ	1	FP02-3	37.500	225
F02 4	560042 524	024422 455	27.070	4 745	4200	0 1	0	FP02-2	37.500	225
FU2-1	568842.524	834422.455	37.870	1./15	1200		1	FP02-2	36.155	225
						o	0	FP02-1	36.155	225

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Ltd.	Network: Foul Network	
Finisklin Business Park	Patrick Carr	
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Node	Easting (m)	Northing (m)	CL (m)	Depth (m)	Dia (mm)	Connection	s	Link	IL (m)	Dia (mm)
F02	568836.581	834404.886	36.470	1.295	1200	. 1	1	FP02-1	35.175	225
							2	FP03	35.175	225
						2	0	FP02	35.175	225
F01-2	568801.338	834447.071	36.145	1.445	1200					
						\bigcirc				
						ò	0	FP01-2	34.700	150
F01-1	568804.667	834431.126	36.130	1.620	1200	1	1	FP01-2	34.510	150
						Q,				
						0	0	FP01-1	34.510	150
F01	568823.495	834416.093	36.315	2.155	1200	4	1	FP01-1	34.160	150
							2	FP02	34.760	225
						0 2	0	FP01	34.160	225
8403	568822.647	834415.009	36.280	3.313	1200	. 1	1	FP01	34.145	225
							2	FP9302	32.967	300
						2	0	8403 EXG	32.967	300
8401	568750.538	834472.521	34.374	3.200	1200		1	8403_EXG	31.174	300

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	•	
	Jennings O'Donnovan and Partners Ltd. Finisklin Business Park Co. Sligo	Jennings O'Donnovan and PartnersFile: 6736-Caltragh-V2.1.pfdLtd.Network: Storm NetworkFinisklin Business ParkPatrick CarrCo. Sligo21/03/2024

Design Settings

Rainfall Methodology	FSR	Maximum Time of Concentration (mins)	30.00
Return Period (years)	100	Maximum Rainfall (mm/hr)	50.0
Additional Flow (%)	20	Minimum Velocity (m/s)	0.75
FSR Region	Scotland and Ireland	Connection Type	Level Inverts
M5-60 (mm)	16.200	Minimum Backdrop Height (m)	0.600
Ratio-R	0.265	Preferred Cover Depth (m)	0.900
CV	0.750	Include Intermediate Ground	\checkmark
Time of Entry (mins)	5.00	Enforce best practice design rules	\checkmark

Adoptable Manhole Type

Max Width (mm)	Diameter (mm)	Max Width (mm)	Diameter (mm)
374	1200	749	1500
499	1350	900	1800

>900 Link+900 mm

Max Depth (m)	Diameter (mm)	Max Depth (m)	Diameter (mm)
1.500	1050	99.999	1200

Circular Link Type

Shape	Circular	Auto Increment (mm)	75
Barrels	1	Follow Ground	х

Available Diameters (mm)

100 150

<u>Nodes</u>

Name	Area (ha)	T of E (mins)	Cover Level (m)	Diameter (mm)	Easting (m)	Northing (m)	Depth (m)
S_9103		5.00	50.502	1200	568899.732	834150.354	3.240
S_9207			50.641	1200	568914.681	834210.426	3.610
S_9205			49.077	1200	568898.307	834221.130	3.051
S_9204			46.360	1200	568909.462	834257.543	6.020
S_9305			42.680	1200	568928.844	834321.595	2.399
S_9303			39.000	1200	568877.627	834363.702	4.707
S08-2	0.036	5.00	46.770	1200	568868.924	834223.994	0.835
S08-1	0.021	5.00	46.430	1200	568894.260	834230.799	0.825
S08	0.051	5.00	46.500	1200	568905.473	834227.921	1.100
S07	0.077	5.00	44.230	1200	568925.150	834292.343	1.270
S07-3	0.179	5.00	41.895	1200	568848.350	834282.383	1.315
S07-2	0.011	5.00	43.490	1200	568905.526	834296.505	3.490
S07-1	0.011	5.00	43.900	1200	568915.936	834294.297	4.110
S06	0.053	5.00	42.370	1200	568925.098	834331.047	3.900
S05-3	0.251	5.00	45.500	1200	568913.516	834464.046	2.610
S05-2	0.212	5.00	41.640	1200	568913.075	834413.129	1.900
S05-1	0.065	5.00	40.640	1200	568911.016	834357.335	2.400
S05	0.027	5.00	40.500	1200	568903.445	834348.218	2.800
S04-2	0.104	5.00	37.350	1200	568832.254	834347.672	1.160
S04-1			39.080	1200	568878.092	834358.888	3.530
S04	0.076	5.00	39.250	1200	568883.881	834365.176	3.980
S03-5	0.122	5.00	43.595	1200	568804.362	834542.538	1.190

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Ltd.	Network: Storm Network	
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				<u>Nodes</u>			
Name	Area (ha)	T of E (mins)	Cover Level (m)	Diameter (mm)	Easting (m)	Northing (m)	Depth (m)
S03-4	0.015	5.00	45.210	1200	568857.017	834519.258	3.000
S03-3-1	0.094	5.00	40.655	1200	568794.570	834510.728	1.620
S03-3	0.087	5.00	44.760	1200	568854.760	834511.209	6.180
S03-2	0.139	5.00	41.450	1200	568843.766	834469.709	3.450
S03-1	0.045	5.00	37.740	1200	568844.031	834421.140	2.090
S03-1-2	0.031	5.00	36.135	1200	568803.434	834446.484	1.160
S03-1-1	0.036	5.00	36.160	1200	568806.563	834431.760	1.270
S03			36.530	1200	568838.866	834404.134	2.030
S02			36.500	1200	568833.100	834396.900	2.160
S_8404			36.260	1200	568822.455	834413.658	2.898
S01			36.780	1200	568832.682	834386.144	2.630

<u>Links</u>

Name	US	DS	Length	ks (mm) /	US IL	DS IL	Fall	Slope	Dia	T of C	Rain
	Node	Node	(m)	n	(m)	(m)	(m)	(1:X)	(mm)	(mins)	(mm/hr)
SP9103	S_9103	S_9207	61.904	0.600	47.262	47.031	0.231	268.0	450	5.83	50.0
SP9207	S_9207	S_9205	19.562	0.600	47.031	46.026	1.005	19.5	450	5.90	50.0
SP9205	S_9205	S_9204	38.083	0.600	46.026	40.340	5.686	6.7	450	5.99	50.0
SP9204	S_9204	S_9305	66.920	0.600	40.340	40.281	0.059	1134.2	450	7.86	50.0
SP9305	S_9305	S_9303	66.304	0.600	40.281	34.293	5.988	11.1	450	8.04	50.0
SP9303	S_9303	S_8404	74.428	0.600	34.293	33.362	0.931	79.9	525	8.53	50.0
SP08-2	S08-2	S08-1	26.234	0.600	45.935	45.605	0.330	79.5	225	5.30	50.0
SP08-1	S08-1	S08	11.576	0.600	45.605	45.400	0.205	56.5	225	5.41	50.0
SP08	S08	S07	67.360	0.600	45.400	42.960	2.440	27.6	225	5.86	50.0
SP07	S07	S07-1	9.419	0.600	42.960	42.485	0.475	19.8	225	5.91	50.0
SP07-3	S07-3	S07-2	58.894	0.600	40.580	40.000	0.580	101.5	300	5.63	50.0
SP07-2	S07-2	S07-1	10.642	0.600	40.000	39.790	0.210	50.7	300	5.71	50.0
SP07-1	S07-1	S06	37.875	0.600	39.790	38.470	1.320	28.7	300	6.13	50.0
SP06	S06	S05	27.635	0.600	38.470	38.240	0.230	120.2	300	6.45	50.0
SP05-3	S05-3	S05-2	50.919	0.600	42.890	40.340	2.550	20.0	225	5.29	50.0
SP05-2	S05-2	S05-1	55.832	0.600	39.740	38.240	1.500	37.2	300	5.65	50.0

Name	Vel	Сар	Flow	US	DS	Σ Area	Σ Add	Pro	Pro
	(m/s)	(I/s)	(I/s)	Depth	Depth	(ha)	Inflow	Depth	Velocity
				(m)	(m)		(I/s)	(mm)	(m/s)
SP9103	1.237	196.7	0.0	2.790	3.160	0.000	0.0	0	0.000
SP9207	4.624	735.4	0.0	3.160	2.601	0.000	0.0	0	0.000
SP9205	7.892	1255.1	0.0	2.601	5.570	0.000	0.0	0	0.000
SP9204	0.595	94.6	0.0	5.570	1.949	0.000	0.0	0	0.000
SP9305	6.135	975.7	0.0	1.949	4.257	0.000	0.0	0	0.000
SP9303	2.506	542.6	0.0	4.182	2.373	0.000	0.0	0	0.000
SP08-2	1.468	58.4	5.9	0.610	0.600	0.036	0.0	48	0.946
SP08-1	1.744	69.3	9.3	0.600	0.875	0.057	0.0	55	1.217
SP08	2.499	99.4	17.6	0.875	1.045	0.108	0.0	64	1.895
SP07	2.951	117.4	30.1	1.045	1.190	0.185	0.0	78	2.487
SP07-3	1.560	110.3	29.1	1.015	3.190	0.179	0.0	105	1.321
SP07-2	2.213	156.5	30.9	3.190	3.810	0.190	0.0	90	1.734
SP07-1	2.946	208.2	62.8	3.810	3.600	0.386	0.0	113	2.591
SP06	1.433	101.3	71.4	3.600	1.960	0.439	0.0	186	1.548
SP05-3	2.941	116.9	40.8	2.385	1.075	0.251	0.0	92	2.689
SP05-2	2.585	182.7	75.3	1.600	2.100	0.463	0.0	134	2.463

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	Jenni Ltd. Finisł Co. S	ngs O'Don klin Busine ligo	novan anc ss Park	l Partners	File: 673 Networ Patrick 21/03/2	File: 6736-Caltragh-V2.1.pfd Network: Storm Network Patrick Carr 21/03/2024				Page 3					
						<u> </u>									
Na	ame	US	DS	Length	ks (mm) /	US IL	DS IL	Fall	Slope	Dia	T of C	Rain			
		Node	Node	(m)	n	(m)	(m)	(m)	(1:X)	(mm)	(mins)	(mm/hr)			
SP0	5-1	S05-1	S05	11.851	0.600	38.240	37.700	0.540	21.9	300	5.71	50.0			
SP0	5	S05	S04	25.891	0.600	37.700	36.900	0.800	32.4	300	6.60	50.0			
SPO	4-2	S04-2	S04-1	47.190	0.600	36.190	35.550	0.640	73.7	300	5.43	50.0			
SPO	4-1	S04-1	S04	8.547	0.600	35.550	35.270	0.280	30.5	300	5.48	50.0			
SPO	4	S04	S03	59.532	0.600	35.270	35.070	0.200	297.7	525	7.37	50.0			
SPO	3-5	S03-5	S03-4	57.572	0.600	42.405	42.210	0.195	295.2	225	6.27	50.0			
SP0	3-4	S03-4	S03-3	8.359	0.600	42.210	42.180	0.030	278.6	225	6.45	50.0			
SP0	3-3-1	S03-3-1	S03-3	60.192	0.600	39.035	38.580	0.455	132.3	225	5.88	50.0			
SP0	3-3	S03-3	S03-2	42.932	0.600	38.580	38.000	0.580	74.0	300	6.84	50.0			
SP0	3-2	S03-2	S03-1	48.570	0.600	38.000	36.250	1.750	27.8	300	7.11	50.0			
SP0	3-1	S03-1	S03	17.773	0.600	35.650	35.350	0.300	59.2	300	7.26	50.0			
SP0	3-1-2	S03-1-2	S03-1-1	15.053	0.600	34.975	34.890	0.085	177.1	300	5.21	50.0			
SP0	3-1-1	S03-1-1	S03	42.505	0.600	34.890	34.500	0.390	109.0	300	5.68	50.0			
SP0	3	S03	S02	9.251	0.600	34.500	34.340	0.160	57.8	450	7.43	50.0			
SPO	2	S02	S01	10.764	0.600	34.340	34.150	0.190	56.7	450	7.49	50.0			

Name	Vel	Сар	Flow	US	DS	Σ Area	Σ Add	Pro	Pro
	(m/s)	(I/s)	(I/s)	Depth	Depth	(ha)	Inflow	Depth	Velocity
				(m)	(m)		(I/s)	(mm)	(m/s)
SP05-1	3.370	238.2	85.9	2.100	2.500	0.528	0.0	125	3.108
SP05	2.773	196.0	161.7	2.500	2.050	0.994	0.0	208	3.083
SP04-2	1.833	129.5	16.9	0.860	3.230	0.104	0.0	73	1.280
SP04-1	2.856	201.9	16.9	3.230	3.680	0.104	0.0	58	1.757
SP04	1.293	279.9	190.9	3.455	0.935	1.174	0.0	319	1.387
SP03-5	0.756	30.0	19.8	0.965	2.775	0.122	0.0	134	0.807
SP03-4	0.778	30.9	22.3	2.775	2.355	0.137	0.0	142	0.846
SP03-3-1	1.135	45.1	15.3	1.395	5.955	0.094	0.0	90	1.027
SP03-3	1.829	129.3	51.7	5.880	3.150	0.318	0.0	132	1.729
SP03-2	2.995	211.7	74.3	3.150	1.190	0.457	0.0	122	2.738
SP03-1	2.046	144.6	81.6	1.790	0.880	0.502	0.0	161	2.106
SP03-1-2	1.178	83.3	5.0	0.860	0.970	0.031	0.0	50	0.655
SP03-1-1	1.505	106.4	10.9	0.970	1.730	0.067	0.0	64	0.977
SP03	2.677	425.8	283.5	1.580	1.710	1.743	0.0	269	2.858
SP02	2.705	430.2	283.5	1.710	2.180	1.743	0.0	268	2.881

Pipeline Schedule

Link	Length	Slope	Dia	Link	US CL	US IL	US Depth	DS CL	DS IL I	OS Depth
	(m)	(1:X)	(mm)	Туре	(m)	(m)	(m)	(m)	(m)	(m)
SP9103	61.904	268.0	450	Circular	50.502	47.262	2.790	50.641	47.031	3.160
SP9207	19.562	. 19.5	450	Circular	50.641	47.031	3.160	49.077	46.026	2.601
SP9205	38.083	6.7	450	Circular	49.077	46.026	2.601	46.360	40.340	5.570
SP9204	66.920	1134.2	450	Circular	46.360	40.340	5.570	42.680	40.281	1.949
SP9305	66.304	11.1	450	Circular	42.680	40.281	1.949	39.000	34.293	4.257
	Link	US	Dia	Node	MH	DS	Dia	Node	MH	
		Node	(mm)	Туре	Туре	Node	e (mm)	Туре	Туре	
	SP9103	S_9103	1200	Manhole	Adoptable	e S_920	7 1200	Manhole	Adoptab	le

SP9207	S_9207	1200	Manhole	Adoptable	S_9205	1200	Manhole	Adoptable
SP9205	S_9205	1200	Manhole	Adoptable	S_9204	1200	Manhole	Adoptable
SP9204	S_9204	1200	Manhole	Adoptable	S_9305	1200	Manhole	Adoptable
SP9305	S_9305	1200	Manhole	Adoptable	S_9303	1200	Manhole	Adoptable

Jen	nings	O'Donnc	ovan and	Partners	s File: (6736-Caltr	agh-V2.1	.pfd	Page 4		
Ltd					Netw	ork: Storn	n Networ	К			
Fini	isklin E	Business	Park		Patrio	ck Carr					
Co.	Sligo				21/03	3/2024					
					<u>P</u>	ipeline Sch	nedule				
Lir	nk	Length	Slope	Dia	Link	US CL	US IL	US Depth	DS CL	DS IL I	DS Depth
6000		(m)	(1:X)	(mm)	Туре	(m)	(m)	(m)	(m)	(m)	(m)
5P93	503	74.428	79.9	525	Circular	39.000	34.293	4.182	36.260	33.362	2.373
SP08	3-2	26.234	/9.5	225	Circular	46.770	45.935	0.610	46.430	45.605	0.600
SP08	8-1	11.576	56.5	225	Circular	46.430	45.605	0.600	46.500	45.400	0.875
SP08	3	67.360	27.6	225	Circular	46.500	45.400	0.875	44.230	42.960	1.045
SP07		9.419	19.8	225	Circular	44.230	42.960	1.045	43.900	42.485	1.190
SP07	'-3	58.894	101.5	300	Circular	41.895	40.580	1.015	43.490	40.000	3.190
SP07	/-2	10.642	50.7	300	Circular	43.490	40.000	3.190	43.900	39.790	3.810
SP07	'-1	37.875	28.7	300	Circular	43.900	39.790	3.810	42.370	38.470	3.600
SP06	5	27.635	120.2	300	Circular	42.370	38.470	3.600	40.500	38.240	1.960
SP05	5-3	50.919	20.0	225	Circular	45.500	42.890	2.385	41.640	40.340	1.075
SP05	5-2	55.832	37.2	300	Circular	41.640	39.740	1.600	40.640	38.240	2.100
SP05	5-1	11.851	21.9	300	Circular	40.640	38.240	2.100	40.500	37.700	2.500
SP05	5	25.891	32.4	300	Circular	40.500	37.700	2.500	39.250	36.900	2.050
SP04	l-2	47.190	73.7	300	Circular	37.350	36.190	0.860	39.080	35.550	3.230
SP04	-1	8.547	30.5	300	Circular	39.080	35.550	3.230	39.250	35.270	3.680
SP04	ļ	59.532	297.7	525	Circular	39.250	35.270	3.455	36.530	35.070	0.935
SP03	8-5	57.572	295.2	225	Circular	43.595	42.405	0.965	45.210	42.210	2.775
SP03	8-4	8.359	278.6	225	Circular	45.210	42.210	2.775	44.760	42.180	2.355
SP03	8-3-1	60.192	132.3	225	Circular	40.655	39.035	1.395	44.760	38.580	5.955
SP03	8-3	42.932	74.0	300	Circular	44.760	38.580	5.880	41.450	38.000	3.150
SP03	8-2	48.570	27.8	300	Circular	41.450	38.000	3.150	37.740	36.250	1.190
SP03	8-1	17.773	59.2	300	Circular	37.740	35.650	1.790	36.530	35.350	0.880
SP03	8-1-2	15.053	177.1	300	Circular	36.135	34.975	0.860	36.160	34.890	0.970
SP03	8-1-1	42.505	109.0	300	Circular	36.160	34.890	0.970	36.530	34.500	1.730
SP03	}	9.251	57.8	450	Circular	36.530	34.500	1.580	36.500	34.340	1.710
	Li	nk	US	Dia	Node	МН	DS	6 Dia	Node	МН	
			Node	(mm)	Туре	Туре	Noc	de (mm)	Туре	Туре	
	SP93	303 S	_9303	1200	Manhole	Adoptab	le S_84	04 1200	Manhole	e Adoptal	ble
	SPO	8-2 S	608-2	1200	Manhole	Adoptab	le S08-	1 1200	Manhole	e Adoptal	ble
	500	01 0	00 1	1200	Manhala	Adaptab	0.0	1200	Manhala	Adaptal	blo

	Noue	(11111)	Type	Type	Noue	(11111)	Type	Type
SP9303	S_9303	1200	Manhole	Adoptable	S_8404	1200	Manhole	Adoptable
SP08-2	S08-2	1200	Manhole	Adoptable	S08-1	1200	Manhole	Adoptable
SP08-1	S08-1	1200	Manhole	Adoptable	S08	1200	Manhole	Adoptable
SP08	S08	1200	Manhole	Adoptable	S07	1200	Manhole	Adoptable
SP07	S07	1200	Manhole	Adoptable	S07-1	1200	Manhole	Adoptable
SP07-3	S07-3	1200	Manhole	Adoptable	S07-2	1200	Manhole	Adoptable
SP07-2	S07-2	1200	Manhole	Adoptable	S07-1	1200	Manhole	Adoptable
SP07-1	S07-1	1200	Manhole	Adoptable	S06	1200	Manhole	Adoptable
SP06	S06	1200	Manhole	Adoptable	S05	1200	Manhole	Adoptable
SP05-3	S05-3	1200	Manhole	Adoptable	S05-2	1200	Manhole	Adoptable
SP05-2	S05-2	1200	Manhole	Adoptable	S05-1	1200	Manhole	Adoptable
SP05-1	S05-1	1200	Manhole	Adoptable	S05	1200	Manhole	Adoptable
SP05	S05	1200	Manhole	Adoptable	S04	1200	Manhole	Adoptable
SP04-2	S04-2	1200	Manhole	Adoptable	S04-1	1200	Manhole	Adoptable
SP04-1	S04-1	1200	Manhole	Adoptable	S04	1200	Manhole	Adoptable
SP04	S04	1200	Manhole	Adoptable	S03	1200	Manhole	Adoptable
SP03-5	S03-5	1200	Manhole	Adoptable	S03-4	1200	Manhole	Adoptable
SP03-4	S03-4	1200	Manhole	Adoptable	S03-3	1200	Manhole	Adoptable
SP03-3-1	S03-3-1	1200	Manhole	Adoptable	S03-3	1200	Manhole	Adoptable
SP03-3	S03-3	1200	Manhole	Adoptable	S03-2	1200	Manhole	Adoptable
SP03-2	S03-2	1200	Manhole	Adoptable	S03-1	1200	Manhole	Adoptable
SP03-1	S03-1	1200	Manhole	Adoptable	S03	1200	Manhole	Adoptable
SP03-1-2	S03-1-2	1200	Manhole	Adoptable	S03-1-1	1200	Manhole	Adoptable
SP03-1-1	S03-1-1	1200	Manhole	Adoptable	S03	1200	Manhole	Adoptable
SP03	S03	1200	Manhole	Adoptable	S02	1200	Manhole	Adoptable

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Finisk Co. Sli	in Busines go	ss Park		Patr 21/0	ick Carr 03/2024						
				<u> </u>	Pipeline S	<u>chedule</u>					
Link SP02	Length (m) 10.764	Slope (1:X) 56.7	e Dia (mm) 7 450	Link Type Circular	US CL (m) 36.500	US IL (m) 34.340	US Depth (m) 1.710	DS ((m) 36.78	CL DS IL) (m) 80 34.15	DS De (m	epth) .180
	Link SP02	US Node S02	Dia (mm) 1200	Node Type Manhole	MH Type Adoptab	DS Node le S01	Dia e (mm) 1200	Node Type Manho	e M E Typ Die Adop	H pe table	
				<u>1</u>	Manhole	<u>Schedule</u>					
Node	Eastin (m)	ng	Northing (m)	CL (m)	Depth (m)	Dia (mm)	Connecti	ons	Link	IL (m)	Dia (mm)
S_9103	568899.	732 8	334150.354	50.502	3.240	1200	Ĵ				
								0	SP9103	47.262	450
S_9207	568914.	681 8	334210.426	50.641	3.610	1200	•	1	SP9103	47.031	450
							1	0	SP9207	47.031	450
S_9205	568898.	307 8	334221.130	49.077	3.051	1200	Å	1	SP9207	46.026	450
							1	0	SP9205	46.026	450
S_9204	568909.	462 8	334257.543	46.360	6.020	1200	\oint	1	SP9205	40.340	450
							1	0	SP9204	40.340	450
S_9305	568928.	844 8	334321.595	42.680	2.399	1200	•	1	SP9204	40.281	450
							1	0	SP9305	40.281	450
S_9303	568877.	627 8	334363.702	39.000	4.707	1200	° K	1	SP9305	34.293	450
<u> </u>	50000	004	24222.004	46 770	0.025	1200	1	0	SP9303	34.293	525
508-2	568868.	924 8	334223.994	46.770	0.835	1200					
								0	SP08-2	45.935	225
S08-1	568894.	260 8	334230.799	46.430	0.825	1200	1	1	SP08-2	45.605	225
	F 6 9 6 9 5	470				40.00		0	SP08-1	45.605	225
508	568905.	473 8	334227.921	46.500	1.100	1200	1	1	SP08-1	45.400	225
~~~		4.5.5				4444		0	SP08	45.400	225
507	568925.	150 8	334292.343	44.230	1.270	1200	0 <	1	5208	42.960	225
							1	0	SP07	42.960	225

Jennings O'Donnovan and Partners	File: 6736-Caltragh-V2.1.pfd	Page 6
Ltd.	Network: Storm Network	
Finisklin Business Park	Patrick Carr	
Co. Sligo	21/03/2024	

Node	Easting (m)	Northing (m)	CL (m)	Depth (m)	Dia (mm)	Connection	IS	Link	IL (m)	Dia (mm)
S07-3	568848.350	834282.383	41.895	1.315	1200					
						⊖→0				
							0	SP07-3	40.580	300
S07-2	568905.526	834296.505	43.490	3.490	1200		1	SP07-3	40.000	300
						1 ->0				
							0	SP07-2	40.000	300
S07-1	568915.936	834294.297	43.900	4.110	1200	P P	1	SP07-2	39.790	300
						12	2	SPU7	42.485	225
<u> </u>	560005 000	024224 047	42.270	2 000	4200		0	SP07-1	39.790	300
506	568925.098	834331.047	42.370	3.900	1200	0	1	SP07-1	38.470	300
						1	0	SP06	38.470	300
S05-3	568913.516	834464.046	45.500	2.610	1200	φ				
						o v	0	SP05-3	42.890	225
S05-2	568913.075	834413.129	41.640	1.900	1200		1	SP05-3	40.340	225
						, v	0	SP05-2	39.740	300
S05-1	568911.016	834357.335	40.640	2.400	1200	1	1	SP05-2	38.240	300
					4000		0	SP05-1	38.240	300
505	568903.445	834348.218	40.500	2.800	1200	° 5 _ 1	1	SP05-1	37.700	300
							Z	3800	56.240	500
						2	0	SP05	37.700	300
S04-2	568832.254	834347.672	37.350	1.160	1200	<b>→</b> 0				
							0	SP04-2	36.190	300
S04-1	568878.092	834358.888	39.080	3.530	1200	- 7	1	SP04-2	35.550	300
						_	0	SP04-1	35.550	300
S04	568883.881	834365.176	39.250	3.980	1200		1	SP04-1	35.270	300
						°~	2	SP05	36.900	300
							0	6004	25 270	525
\$03-5	568804 362	83/15/12 538	13 595	1 190	1200		0	5P04	35.270	525
303-3	508804.502	834342.338	43.395	1.190	1200					
							0	SP03-5	42.405	225
S03-4	568857.017	834519.258	45.210	3.000	1200	1	1	SP03-5	42.210	225
						$  \mathcal{Y}$				
						0	0	SP03-4	42.210	225

Jennings O'Donnovan and Partners	File: 6736-Caltragh-V2.1.pfd	Page 7
Ltd.	Network: Storm Network	
Finisklin Business Park	Patrick Carr	
Co. Sligo	21/03/2024	

Node	Easting (m)	Northing (m)	CL (m)	Depth (m)	Dia (mm)	Connection	S	Link	IL (m)	Dia (mm)
S03-3-1	568794.570	834510.728	40.655	1.620	1200					
						()→0				
							0	SP03-3-1	39.035	225
S03-3	568854.760	834511.209	44.760	6.180	1200	2	1	SP03-3-1	38.580	225
						1-	2	SP03-4	42.180	225
						o	0	SP03-3	38.580	300
S03-2	568843.766	834469.709	41.450	3.450	1200	$\phi$	1	SP03-3	38.000	300
	560044.004				4000	, o	0	SP03-2	38.000	300
S03-1	568844.031	834421.140	37.740	2.090	1200	$\Rightarrow$	1	SP03-2	36.250	300
602.4.2	560002 424	024446 404	26 125	1 1 0 0	1200	0	0	SP03-1	35.650	300
503-1-2	568803.434	834446.484	36.135	1.160	1200	P				
						Ő	0	SP03-1-2	34.975	300
S03-1-1	568806.563	834431.760	36.160	1.270	1200		1	SP03-1-2	34.890	300
						0	0	SP03-1-1	34.890	300
S03	568838.866	834404.134	36.530	2.030	1200	2 1. /	1	SP03-1-1	34.500	300
							2	SP03-1	35.350	300
						0 3	3	SP04	35.070	525
\$02	568833 100	831306 000	36 500	2 160	1200		1	SP03	34.500	450
302	508855.100	834390.900	30.300	2.100	1200	$\phi$	Ŧ	3603	54.540	450
<u> </u>	E 60022 4EE	024442.050	26.260	2 000	4200	0 O	0	SP02	34.340	450
5_8404	568822.455	834413.658	36.260	2.898	1200		1	5P9303	33.362	525
S01	568832.682	834386.144	36.780	2.630	1200		1	SP02	34.150	450
			<u>s</u>	imulatior	n Settin	igs				
	Rainfall N Ar Skip	Methodology FSR Region M5-60 (mm) Ratio-R Summer CV Winter CV alysis Speed Steady State	FSR Scotland 16.200 0.265 0.750 0.840 Normal x	and Irela	nd	Drain Down T Additional Stora Check Discha 3 10 Check Dischar	ime ige ( 1 ye 0 ye 0 ye	(mins) 24 (m³/ha) 20 Rate(s) √ ear (l/s) 6. ear (l/s) 13 ear (l/s) 15 /olume x	40 0.0 5 3.0 5.4	

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	Jennings O Do	onnovan	anu Par	thers	File: 6	/30-0	aitragn	-vz.1.più		Page	0	
	Ltd.				Netwo	rk: St	orm Ne	etwork				
	Finisklin Busir	ness Park	<b>&lt;</b>		Patrick	Carr						
	Co. Sligo				21/03/	2024						
					St	orm D	Ouratio	ns				
	15	60	180	360	600	9	960	2160	432	0 720	00	10080
	30	120	240	480	720	1	440	2880	576	0 864	40	
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		Retu	urn Peri	od Clim	ate Cha	nge	Addit	ional Are	a Ado	litional Fl	ow	
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Dro. dou				a-dovolo	nmoi	nt Disc	hargo Rat	<u>م</u>				
				Site Make	eup Gi	reenfi	eld	Growt	h Facto	r 30 year	1.65	5
			Green	ield Metl	nod IH	124		Growth	Factor	100 year	1.96	5
		Positive	lv Drain	ed Area (	'ha) 2	098			Better	nent (%)	0	
			.,	SAAR (n	nm) 12	97				OBar	7 9	
				Soil In	, 12 dov 7				01	voar (I/c)	65	
				301111		20			0.20	year $(1/s)$	12 0	
					5rk U.	50			Q 30	year (1/S)	13.0	J

#### Node S01 Carpark Storage Structure

Region 11

Growth Factor 1 year 0.83

Q 100 year (l/s) 15.4

Base Inf Coefficient (m/hr)	0.03370	Invert Level (m)	33.880	Slope (1:X)	1000.0
Side Inf Coefficient (m/hr)	0.03370	Time to half empty (mins)		Depth (m)	1.700
Safety Factor	2.0	Width (m)	21.500	Inf Depth (m)	
Porosity	0.60	Length (m)	50.000		

# APPECNDIX C

# **TYPICAL PIPE SPECIFICATION**



#### JFC Plastics Ltd

Unit 6 Goldicote Business Park Ettington Stratford-upon-Avon Warwickshire CV37 7NB Tel: 01789 740102 Fax: 01789 740037 email: info@jfc.ie website: www.jfc.ie



Agrément Certificate 02/H069 Product Sheet 1

# JFC CORRIPIPE TWINWALL DRAINAGE SYSTEM

# JFC CORRIPIPE TWINWALL HIGH-DENSITY POLYETHYLENE FILTER AND CARRIER PIPES AND COUPLINGS

This Certificate is issued under the Highway Authorities' Product Approval Scheme (HAPAS) by the British Board of Agrément (BBA) in conjunction with the Highways Agency (HA) (acting on behalf of the overseeing organisations of the Department for Transport; the Scottish Executive; the Welsh Assembly Government and the Department for Regional Development, Northern Ireland), the Association of Directors of Environment, Economy, Planning and Transport (ADEPT), the Local Government Technical Advisers' Group and industry bodies. HAPAS Agrément Certificates are normally each subject to a review every five years.

#### PRODUCT SCOPE AND SUMMARY OF CERTIFICATI

This Certificate relates to JFC CorriPipe Twinwall High-Density Polyethylene Filter and Carrier Pipes and Couplings, in a range of sizes for use in highway drainage for the collection and disposal of surface and sub-surface water.

#### AGRÉMENT CERTIFICATION INCLUDES:

- factors relating to compliance with HAPAS requirements
- factors relating to compliance with Regulations where applicable
- independently verified technical specification
- assessment criteria and technical investigations
- design considerations
- installation guidance
- regular surveillance of production
- formal five-yearly review.

#### **KEY FACTORS ASSESSED**

Strength — the fittings have adequate strength to resist loads associated with installation and service (see section 5).

Performance of joints - the system will remain watertight under normal service conditions (see section 6).

Durability — the system will have a service life in excess of 50 years (see section 10).

The BBA has awarded this Agrément Certificate to the company named above for the system described herein. This system has been assessed by the BBA as being fit for its intended use provided it is installed, used and maintained as set out in this Certificate.

On behalf of the British Board of Agrément

BCChamberter

nA Ceeper

Greg Cooper Chief Executive

Date of First issue: 18 July 2011

Brian Chamberlain Head of Approvals — Engineering

Originally certificated on 28 March 2002 (150 mm, 225 mm and 300 mm) and on 23 December 2005 (375 mm, 450 mm and 600 mm).

The BBA is a UKAS accredited certification body — Number 113. The schedule of the current scope of accreditation for product certification is available in pdf format via the UKAS link on the BBA website at www.bbacerts.co.uk

Readers are advised to check the validity and latest issue number of this Agrément Certificate by either referring to the BBA website or contacting the BBA direct.

British Board of Agrément		tel: 01923 665300
Bucknalls Lane		fax: 01923 665301
Garston, Watford		e-mail: mail@bba.star.co.ul
Herts WD25 9BA	©2011	website: www.bbacerts.co.uk

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# HAPAS Requirements

#### **Requirements**

The general requirements for drains are contained in the Manual of Contract Documents for Highway Works (MCHW), Volume 1.

The general requirements for structural wall pipes and fittings are contained in the MCHW, Volume 1, Clause 518. Further information and guidance is given in the MCHW, Volume 2 and Volume 3 (Drawing Numbers F1 and F2). Additional site requirements may be included on particular contracts.

# Regulations

Construction (Design and Management) Regulations 2007

#### Construction (Design and Management) Regulations (Northern Ireland) 2007

Information in this Certificate may assist the client, CDM co-ordinator, designer and contractors to address their obligations under these Regulations.

See sections:

1 Description (1.3), 2 Delivery and site handling (2.1), 3 General and 11 General of the Installation part of this Certificate.

# General

This Certificate relates to JFC CorriPipe Twinwall 150 mm, 225 mm, 300 mm, 375 mm, 450 mm and 600 mm High-Density Polyethylene Filter and Carrier Pipes and Couplings.

The system is for use in highway drainage for the collection and disposal of surface and sub-surface water in accordance with Highways Agency (HA) requirements, *Manual of Contract Documents for Highway Works* (MCHW), Volume 1, Clause 518, and Volume 2, and the conditions set out in the *Design Considerations* and *Installation* parts of this Certificate.

# **Technical Specification**

#### 1 Description

1.1 JFC CorriPipe Twinwall 150 mm, 225 mm, 300 mm, 375 mm, 450 mm and 600 mm Diameter High-Density Polyethylene Filter and Carrier Pipes and Couplings are manufactured from a blended, black polyethylene by a twin extrusion process. The two high-density polyethylene pipes are extruded simultaneously, one inside the other, and heat-welded together in one continuous process.

1.2 The products tested and covered by this Certificate are manufactured from material with the specification given in Table 1.

Table 1 Material properties/specification								
Property	Test method reference	Specification						
Tensile properties	EN 638, ISO 527-2	≥18 MPa						
Oxygen induction time Melt flow rate	EN 728 ISO 1133	≥4 mins ≤0.75 g (10 mins) 2.16 kg at 190°C						
Density	ISO 1183-3	≥935 kg·m ⁻³						
Heat reversion	ISO 12091	N/A						

1.3 The outer wall is corrugated and the inner wall is smooth finished. Details and dimensions are given in Table 2 and Figure 1.

Table 2 Pipe dimensions								
Nominal internal pipe diameter	External pipe diameter	t ₁ min	t ₂ min	t ₃ min	Nominal length	Nominal weight	Pitch	
d ₁ (mm)	$d_2^{}$ (mm)	(mm)	(mm)	(mm)	(m)	(kg·m⁻¹)	(mm)	
150	178 ± 1.5	0.8	1.8	1.0	6	1.4	20.0	
225	265 ± 2	0.9	2.6	1.2	6	3.0	25.5	
300	$354 \pm 2.5$	1.2	3.0	1.5	6	5.0	31.0	
375	$426 \pm 3$	1.5	3.5	1.5	6	6.0	39.9	
450	$512 \pm 3$	1.8	4.0	1.5	6	8.5	50.1	
600	$680 \pm 4$	2.1	4.3	1.8	6	14.5	66.9	

#### Figure 1 Twinwall pipe



1.4 Black polypropylene couplings are available for the 150 mm, 225 mm and 300 mm sizes of pipe and black polyethylene for the 375 mm, 450 mm and 600 mm sizes (see Table 3 and Figure 2).

Table 3 Coup	ling dimensic	ons		
Nominal	Internal	diameter	Nominal	Nominal seal
internal/external pipe diameter (mm)	at first dwell (mm)	at first dwell at second dwell (mm) (mm)		height (mm)
150/178	178	179	180	17.0
225/265	268	269	220	25.5
300/354	356	356.5	245	32.0
375/426	429	429	321	32.5
450/512	514	514	390	36.9
600/680	686	686	675	49.0
(1) T	In I of			

(1) Tapered along coupling length.



1.5 Each coupling requires two rubber seals which are manufactured to BS EN 681-1 : 1996 (see Figure 3). The seals must be fitted in accordance with the installation instructions to ensure a watertight joint.



1.6 Pipes can be supplied either slotted or unslotted. Slotted pipe is available with the slots in the dwell between corrugations equally spaced around the circumference (see Table 4 and Figure 4).

Table 4 Si	lotted pipe dete	ails			
Internal pipe diameter (nominal)	No of slots per alternate	No of dwells per metre	Slot length	Slot width	Permeable area (minimum)
(mm)	dwell		(mm)	(mm)	(mm ² ·m ⁻¹ )
150	4	51	15 to 20	2.0 to 2.5	6120
225	4	39	15 to 30	2.0 to 2.5	4680
300	4	32	20 to 40	2.0 to 2.5	5120
375	3	25	42 to 85	2.7 to 3.3	4263
450	3	20	48 to 85	2.8 to 3.5	4024
600	3	15	76 to 106	2.9 to 3.5	4942

Figure 4 Details of slots (optional)



1.7 Continuous quality control is exercised during manufacture. Checks include:

#### Pipes

- dimensional accuracy
- impact resistance
- short-term stiffness

#### Couplings

• dimensional accuracy/visual check.

1.8 A label bearing the BBA identification mark is attached to each pipe length and fitting or to each pack of pipes.

# 2 Delivery and site handling

2.1 Handling, storage and transportation should be in accordance with BS 5955-6 : 1980.

2.2 When long-term storage is envisaged, JFC CorriPipe twinwall slotted and unslotted pipes and couplings should be protected from direct sunlight. If protection cannot be provided, consideration must be given to the effects of daily exposure to direct sunlight:

- up to 3 months negligible UV degradation but possible extreme surface temperatures of up to 80°C may cause some localised distortion
- 3 months to 12 months may have significant effect on the impact resistance and physical properties
- over 12 months damage will occur unless protection provided.
- 2.3 The manufacturer has the option of adding chemicals to provide enhanced UV stability on request.

2.4 Pipes are generally delivered in prepacked bundles and should be retained in their packaging until installation.

# Assessment and Technical Investigations

The following is a summary of the assessment and technical investigations carried out on JFC CorriPipe Twinwall 150 mm, 225 mm, 300 mm, 375 mm, 450 mm and 600 mm Diameter High-Density Polyethylene Filter and Carrier Pipes and Couplings.

# Design Considerations

#### 3 General

JFC CorriPipe Twinwall 150 mm, 225 mm, 300 mm, 375 mm, 450 mm and 600 mm Diameter High-Density Polyethylene Filter and Carrier Pipes and Couplings comply with the requirements of the Highways Agency (HA) *Manual of Contract Documents for Highway Works* (MCHW), Volume 1, Clause 518.5 for pipe, Clause 518.6 for couplings and Clause 518.7 for the system, and is suitable for use in highways for the collection and disposal of surface and sub-surface water.

#### 4 Practicability of installation

The pipes are installed using traditional drain-laying methods in accordance with HA requirements and the MCHW, Volume 1, Clauses 503, 505, 518.7 and 518.8. Due to the lightweight nature of the pipe material, handling and jointing are easily performed.

#### 5 Strength

5.1 The pipes have a ring stiffness in excess of 6 kN·m⁻², a creep ratio of less than 4 and adequate resistance to static loads.

5.2 The pipes have adequate resistance to impact loads to which they may be subjected during installation and in service.

#### 6 Performance of joints

6.1 Joints on filter pipes made from pipe and couplings without the rubber seals are not partially watertight as defined in the MCHW, Volume 1, Clause 504.3.

6.2 Correctly made, the joints constructed from pipe and couplings with rubber seals remain watertight when subjected to deflection and distortion, and comply with the MCHW, Volume 1, Clauses 504.3 and 518.7 (see section 14).

## 7 Water infiltration

The slot area for the pipes exceeds the minimum requirement of 1000 mm² per metre length as given in the MCHW, Volume 1, Clause 518.3 (see Table 3).

#### 8 Flow characteristics

8.1 The pipes will have normal flow characteristics associated with thermoplastics pipes.

8.2 Full-bore velocities are available from the *Tables for the Hydraulic Design of Pipes, Sewers and Channels,* Volume 2, 8th Edition, by H R Wallingford and D I H Barr. Appropriate values are based on the Colebrook-White equation. An appropriate value of roughness coefficient should be selected when designing the drainage system. For new pipes, a value of 0.006 is applicable, but for designs a value of 0.6 is generally used.

#### 9 Maintenance

9.1 The slots are designed to restrict the ingress of silt into the drains.

9.2 Access to the system for cleaning should be provided by conventional methods.

9.3 The system can be rodded using flexible drain rods. In common with other standard plastic drainage systems, toothed root cutters and rods with metal ferrules, as used with some mechanical clearing systems, could damage the pipes and couplings and should not be used.

9.4 Tests indicate that the pipes have adequate resistance to cleansing using pressure jetting equipment (see section 13.1). It is recommended that low-pressure, high-volume systems are utilised in accordance with MCHW, Clause 520.

# 10 Durability

In the opinion of the BBA, the material from which the pipes and couplings are manufactured will not significantly deteriorate and the anticipated life of the system will be in excess of 50 years.

# Installation

# 11 General

11.1 JFC CorriPipe Twinwall 150 mm, 225 mm, 300 mm, 375 mm, 450 mm and 600 mm Diameter High-Density Polyethylene Filter and Carrier Pipes and Couplings must be installed in accordance with HA requirements and the MCHW, Volume 1, Clauses 503, 505, 518.7 and 518.8.

11.2 The pipes and couplings must be protected against damage from site construction traffic.

# 12 Procedures

12.1 For typical laying, trench and backfilling specification details, reference should be made to Figure 5 and the MCHW, Volume 3, Drawings No F1 (Type T and S) and No F2 (Type G, H and I).

Figure 5 Installation details



12.2 Pipes are cut easily using conventional hand tools, and should be cut square between the corrugations.

12.3 For a watertight joint, the pipe ends and coupling should be cleaned and the rubber seal fitted externally in the first or second dwell. The seal and inside of the coupling should be lubricated and the pipe pushed fully home to the central register either by hand, or using a lever if necessary.

12.4 Care should be taken during backfill to maintain the line and level of the pipeline. If necessary, the pipe should be restrained to prevent uplift.

# Technical Investigations

## 13 Tests

13.1 Tests were carried out on the pipe in accordance with the MCHW, Volume 1, Clause 518.5 to determine:

- ring stiffness to BS EN ISO 9969 : 1995
- creep ratio to BS EN ISO 9967 : 1995
- longitudinal bending to the MCHW, Volume 1, Clause 518.11
- rodding resistance to the MCHW, Volume 1, Clause 518.12
- impact resistance at 0°C and 23°C to BS EN 1411 : 1996 with a striker of 1.0 kg mass and 25 mm diameter conical head
- water jetting WRc method.

13.2 Tests were carried out on the system to establish:

- leaktightness of joint to BS EN 1277 : 2003, Method 4, Conditions A, B and C
- insertion force (ease of jointing).

13.3 Tests were carried out to establish the dimensional accuracy of the pipe, coupling and ring seal.

## 14 Investigations

14.1 An examination was made of data in relation to the effect of the production tolerances on the performance of the products.

14.2 An evaluation of existing data was made to assess material properties, chemical resistance and durability.

14.3 Calculations were carried out to determine slot area.

14.4 The manufacturing process was examined, including the methods adopted for quality control, and details were obtained of the quality and composition of the materials used.

# Bibliography

BS 5955-6 : 1980 Plastics pipework (thermoplastics materials) — Code of practice for the installation of unplasticized PVC pipework for gravity drains and sewers

BS EN 681-1 : 1996 Elastomeric seals — Material requirements for pipe joint seals used in water and drainage applications — Vulcanized rubber

BS EN 763 : 1995 Plastics piping and ducting systems — Injection-moulded thermoplastics fittings — Test method for visually assessing effects of heating

BS EN 1277 : 2003 Plastics piping systems — Thermoplastics piping systems for buried non-pressure applications — Test methods for leaktightness of elastomeric sealing ring type joints

BS EN 1411 : 1996 Plastics piping and ducting systems — Thermoplastics pipes — Determination of resistance to external blows by the staircase method

BS EN ISO 9967 : 1995 Thermoplastics pipes - Determination of creep ratio

BS EN ISO 9969 : 1995 Thermoplastics pipes - Determination of ring stiffness

EN 638 : 1994 Plastics piping and ducting systems — Thermoplastics pipes — Determination of tensile properties EN 728 : 1997 Plastics piping and ducting systems — Polyolefin pipes and fittings — Determination of oxidation induction time

ISO 527-2 : 1993 Plastics — Determination of tensile properties — Test conditions for moulding and extrusion plastics ISO 1133 : 1997 Plastics — Determination of the melt mass-flow rate (MFR) and the melt volume-flow rate (MVR) of thermoplastics

ISO 1183-3 : 1999 Plastics — Methods for determining the density of non-cellular plastics — Gas pyknometer method

ISO 4440-1 : 1994 Thermoplastics pipes and fittings — Determination of melt mass-flow rate — Test method

ISO 4451 : 1980 Polyethylene (PE) pipes and fittings — Determination of reference density of uncoloured and black polyethylenes

ISO 12091 : 1995 Structural wall thermoplastics pipes - Oven test

Manual of Contract Documents for Highway Works, Volume 1 Specification for Highway Works, August 1998 (as amended)

Manual of Contract Documents for Highway Works, Volume 2 Notes for Guidance on the Specification for Highway Works, August 1998 (as amended)

Manual of Contract Documents for Highway Works, Volume 3 Highway Construction Details, March 1998 (as amended)

# Conditions of Certification

### 15 Conditions

15.1 This Certificate:

- relates only to the product/system that is named and described on the front page
- is issued only to the company, firm, organisation or person named on the front page no other company, firm, organisation or person may hold or claim that this Certificate has been issued to them
- is valid only within the UK
- has to be read, considered and used as a whole document it may be misleading and will be incomplete to be selective
- is copyright of the BBA
- is subject to English Law.

15.2 Publications, documents, specifications, legislation, regulations, standards and the like referenced in this Certificate are those that were current and/or deemed relevant by the BBA at the date of issue or reissue of this Certificate.

15.3 This Certificate will remain valid for an unlimited period provided that the product/system and its manufacture and/or fabrication, including all related and relevant parts and processes thereof:

- are maintained at or above the levels which have been assessed and found to be satisfactory by the BBA
- continue to be checked as and when deemed appropriate by the BBA under arrangements that it will determine
- are reviewed by the BBA as and when it considers appropriate
- remain in accordance with the requirements of Highway Authorities' Product Approval Scheme.
- 15.4 The BBA has used due skill, care and diligence in preparing this Certificate, but no warranty is provided.

15.5 In issuing this Certificate, the BBA is not responsible and is excluded from any liability to any company, firm, organisation or person, for any matters arising directly or indirectly from:

- the presence or absence of any patent, intellectual property or similar rights subsisting in the product/system or any other product/system
- the right of the Certificate holder to manufacture, supply, install, maintain or market the product/system
- individual installations of the product/system, including their nature, design, methods, performance, workmanship and maintenance
- any works and constructions in which the product/system is installed, including their nature, design, methods, performance, workmanship and maintenance
- any loss or damage, including personal injury, howsoever caused by the product/system, including its manufacture, supply, installation, use, maintenance and removal.

15.6 Any information relating to the manufacture, supply, installation, use, maintenance and removal of this product/ system which is contained or referred to in this Certificate is the minimum required to be met when the product/system is manufactured, supplied, installed, used, maintained and removed. It does not purport in any way to restate the requirements of the Health and Safety at Work etc. Act 1974, or of any other statutory, common law or other duty which may exist at the date of issue or reissue of this Certificate; nor is conformity with such information to be taken as satisfying the requirements of the 1974 Act or of any statutory, common law or other duty of care.

British Board of Agrément
Bucknalls Lane
Garston, Watford
Herts WD25 9BA

tel: 01923 665300 fax: 01923 665301 e-mail: mail@bba.star.co.uk website: www.bbacerts.co.uk

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# CorriPipe[™] Technical Specification





# 1. Introduction

CorriPipe[™] is a twin wall high density polyethylene pipe manufactured from a blended black polyethylene by a twin extrusion process.

Two high density polyethylene walls are extruded simultaneously, one inside the other, and heat-welded together in one continuous process. The outer wall is corrugated and the inner wall is smooth finished.

It is a combination of the corrugations, and the heatwelding of the two walls, that give CorriPipe[™] its excellent structural strength while its smooth inner wall ensures increased flow capacity.

Its applications include surface and storm water drainage in civil engineering, construction, sports amenity, agricultural and other sub-soil applications.

CorriPipe[™] is fully BBA (British Board of Agrément) approved and HAPAS (Highways Agency Product Approval Scheme) certified.



Figure 1. – CorriPipe™

# 2. Dimensions

CorriPipe[™] comes in a complete range between 100m and 600mm and is available in either carrier of filter pipe. CorriPipe[™] also has a complete range of fittings and junctions as detailed below.

Nominal Size	Inside Diameter	Outside Diameter	Pipe Length
(mm)	(mm)	(mm)	(m)
94	94	110	6
150	149	176	6
225	221	265	6
300	295	354	6
375	370	426	6
450	445	512	6
600	590	680	6

	Table 1	. – CorriPi	pe™ Dim	nensions
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Nominal Size	No. of slots	Nom. Slot	Pefrorated Area
(mm)	per alternate dwell	Width (mm)	(mm²/m)
94	4	1.5	7920
150	4	2	6120
225	4	2	4680
300	4	2	5120
375	3	3	4263
450	3	3	4024
600	3	3	4942

#### Table 2. – Perforated Pipe Detail

Note: CorriPipe also available in various perforation specification. e.g. half perforated, double perforated.

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Nominal Size	Code	Fitting Type
(mm)		
150	150TB30	30° Bend
150	150TB45	45° Bend
150	150TB90	90° Bend
150	150TT90	Equal Tee
150	150TY45	Equal Wye
150	150SWSTT90	Single Wall Tee
225	225TB30	30° Bend
225	225TB45	45° Bend
225	225TB90	90° Bend
225	225TT90	Equal Tee
225	225TY45	Equal Wye
225	225/150TT90	Unequal Tee 150
225	225/150TY45	Unequal Wye 150
225	225SWSTT90	Single Wall Tee
300	300TB30	30° Bend
300	300TB45	45° Bend
300	300TB90	90° Bend
300	300TT90	Equal Tee
300	300TY45	Equal Wye
300	300/150TT90	Unequal Tee 150
300	300/150TY45	Unequal Wye 150
300	300/225TT90	Unequal Tee 225
300	300/225TY45	Unequal Wye 225
300	300SWSTT90	Single Wall Tee

Table 3. – CorriPipe[™] Fittings

Note: Larger fitting sizes fabricated on request

# 3. Hydraulic Capacity

There are two main formulas used in hydraulic calculations of gravity flow pipelines – Manning's and Colebrook-White:

#### Manning's

Manning's is the most popular equation for stormwater design because it is simple to apply and it generally provides an acceptable level of accuracy.

$$Q = \frac{1}{n} A R^{\frac{2}{3}} S^{\frac{1}{2}}$$

- Q = Water Discharge [m³/s]
- n = Manning's roughness factor [s/m1/3]]
- A = Cross-sectional area [m2]
- R = Hydraulic radius [m]
- S = Surface Water Slope [m/m]

#### **Colebrook-White**

A more accurate method for calculations involving FRC[™] pipes is to utilize the Colebrook-White formula. The Colebrook-White design chart for FRC[™] should allow quick and easy estimates without involved calculations.

$$V = -2\sqrt{2gDS} \log \left(\frac{k}{3.7D} + \frac{2.51v}{d\sqrt{2gDS}}\right)$$

- V = Velocity (m/s)
- S = Hydraulic gradient (m/m)
- k = Hydraulic roughness (m)
- R = Hydraulic radius = D/4 (m)
- D = Pipe internal diameter (m)
- g = Gravitational acceleration (m/s2)
- v = Kinematic viscosity of water (m2/s)

# 4. Cover Depths

## **Minimum Cover Depths**

JFC Manufacturing Limited recommends the following minimum cover depths.

- 0.6m for non trafficked green areas
- 0.9m to finished surface for trafficked areas <u>not</u> subject to Highways Agency or National Roads Authority requirements
- 1.2m to finished surface for trafficked areas <u>subject</u> to Highways Agency or National Roads Authority requirements.

In certain circumstances lower minimum cover levels may be allowed. e.g. installation with rigid pavement, concrete surround etc. Please contact JFC for more information.

#### Maximum Cover Depths

The maximum cover depth for CorriPipe[™] is normally between 6-10 meters when installed in accordance with series 500 of the MCDHW Volume 1 as detailed in the CorriPipe[™] BBA certificate.

The actual maximum allowable cover level is dependent on the following installation parameters and is often well in excess of 6-10 meters:

- The native soil stiffness
- The pipe bed and surround stiffness
- The size of the trench
- The density of the overburden
- Hydrostatic loading
- Factor of Safety
- Maximum allowable deflection limit

For specific site conditions JFC can calculate the maximum pipe deflection based on the above parameters. Contact JFC for more details.

# 4. Installation

JFC CorriPipe is to be installed in accordance with the following national guidelines. In countries outside that specified contact JFC for more details.

#### Ireland

The Manual of Contract Documents for Road Works, Volume 1 series 500, clauses 503 and 505 as published by the NRA.

#### **United Kingdom**

The Manual of Contract Documents for Highway Works, Volume 1 series 500, clauses 503, 505, 518.7 and 518.8 as published by the Highways Agency

#### **Trench Preparation**

The trench width is generally between OD+300mm and OD+600mm but larger trenches are permissible. The trench should provide for a minimum of 150mm pipe bed and local soft spots must be removed and replaced with hardcore. The pipe must sit evenly on the bed and must be free of voids under the pipe. The trench should not be excavated too far in advance of pipe installation. All trenches are to be excavated in accordance with national health and safety regulations and local building regulations.

#### Sidefill

CorriPipe[™] is to backfilled as described in the MCDHW, Volume 1, Series 500. Sidefill material is dependent on specification but is normally a well graded granular material or small single size aggregate. The pipe surround material must fully support the pipe. Compaction may be required depending on ground conditions and sidefill material used. If compaction is required the compaction equipment must not come in contact with the pipe. The sidefill material should extend to 100mm over the crown of the pipe.

#### Backfill

Backfill is to continue to a minimum of 300mm above the crown of the pipe with suitable material as per specification. The material should be free of any stone particles greater than 50mm. Compaction should not be carried out until a minimum cover of 300mm is achieved. Compaction equipment should be sized so as not to exert any undue stress in the pipe. Further backfill to the required level should be carried out in layer no greater than 300mm.



Figure 2. – Typical Installation Details

- A = Backfill B = Sidefill C = Bed
- D = Earth
- OD = Outside Diameter of Pipe

# **CorriPipe[™] Technical Specification**

# 5. Jointing

CorriPipe[™] is manufactured in 6 meter lengths and is joined with straight couplers or suitable fittings (e.g. tees, wyes, bends etc.)

CorriPipe[™] provides a fully watertight seal when installed in accordance with JFC recommendations.

Leak tightness is in accordance with BS EN 1277:1997. The maximum permitted angular deflection is 2°.

Rubber seals used in watertight applications are in accordance with BS EN 681-1:1996

JFC recommends the following procedure for joining CorriPipe[™] and associated fittings / couplers.

- Cut the pipe to the require length with a conventional handsaw.
- Clean the end of the pipe and accompanying coupler / fitting.
- Install a ring seal in the first dwell of the pipe for watertight joints.
- Ring seals are bi directional
- Lubricate the ring seal and accompanying coupler / fitting.
- Offer the fitting / coupler up to the pipe
- Lever the fitting / coupler onto the pipe with a piece of timber ensuring not to damage the pipe. Larger pipes may require mechanical assistance.
- Ensure the fitting / coupler is butted fully against the pipe.
- For joining pipes to the opposite side of the fitting / coupler follow the same steps as outlined above.



Figure 3. – Typical Joint Details

# 6. Pressure Testing

There are two methods of pressure testing, the air test method and the water test method. The most common method is the air test method and the test procedure is outlined below.

- Block the ends of the pipe / fitting with a suitable expanding stopper, ensuring both plug and pipe are cleaned prior to fitting.
- Fill a U-Tube manometer with water to the correct level, ensuring there are no trapped air bubbles in the water.
- Connect the u-tube to the fitting on the expandable stopper.
- Increase the pressure in the pipe until a head of water of 100mm is reached.
- Allow the pressure to stabilise for a number of minutes, increasing the pressure if it drops.
- Record the pressure drop over a five minute period.
- To pass the test the pressure should not drop below a 75mm head of water.

Note: Temperature has a critical effect on the test, a 1°C change in air temperature inside the pipe is sufficient for the test to fail.

# 6. Transportation, Handling and Storage

#### General

Handling should be done carefully and in accordance with national health and safety guidelines. Dragging of pipes and fittings must be avoided. HDPE pipes and fittings become slippery in wet or in cold weather and extra precautions may be necessary.

Pipes up to 450mm in size are palletised with wooden frames and steel straps. 600mm pipes are generally steel banded in two's but can also be supplied loose.

Nominal Size	Number of Pipes
(mm)	per Pallet
100	100
150	33
225	14
300	8
375	5
450	4
600	2 / steel banded

Table 4. – CorriPipe[™] Pallet Quantities

#### Storage

All materials should be carefully inspected at the time of delivery and any defects should be notified and reported immediately. All pipe stacks should be made on firm, flat ground to support the weight of the pipes and lifting equipment. For safety and Pipes and fittings should be transported and stored in their packaging.

Delivery vehicles should be provided with a clean, flat bed, free from sharp objects. Care must be taken to prevent slippage or excessive bowing of the pipes. Tie the load well to prevent rubbing. Use nylon straps, not chains or ropes.

The stacking height for pipes should be limited to not more than 3 meters. Pipes should be not be stored in open areas subject to high winds.

It is recommended that CorriPipe[™] is not stored in direct sunlight for more than 3 months.







# **Contact Details**

#### **Head Office Ireland**

JFC Manufacturing Co Ltd Weir Road, Tuam Co Galway Ireland **Tel:** (+) 353 93 24066 **Fax:** (+) 353 93 24923 **Email:** info@jfc.ie **Web:** www.jfc.ie

## JFC Plastics Ltd

Unit 6, Goldicote Business Park Ettington, Nr Stratford-upon-Avon, Warweickshire, CV37 7NB, UK

Tel: (+) 44 (0) 1789 740102 Fax: (+) 44 (0) 1789 740037 Email: info@jfcuk.com Web: www.jfcuk.com

## **UK Sales Office**

JFC Manufacturing (Europe) Ltd Maes Y Clawdd Industrial Estate, Maesbury Road, Oswestry, Shropshire, SY10 8NN, UK **Tel:** (+) 44 (0) 1691 659226 **Fax:** (+) 44 (0) 1691 659344 **Email:** info@jfcuk.com **Web:** www.jfcuk.com

## JFC Recycling Division

JFC Plastics Hardwick Road, Astmoor Ind. Estate, Runcorn, Cheshire WA7 1PH **Tel:** + 44 (0) 1928 5833 90 **Fax:** + 44 (0) 1928 580 941 **Email:** info@jfcplastics.com **Web:** www.jfcplastics.com

## **Dutch Sales Office**

JFC Manufacturing (Europe) Ltd De Kamp 2A, 9231 Br Surhuisterveen, Holland **Tel:** (+) 31 (0) 512 366440 **Fax:** (+) 31 (0) 512 360420 **Email:** info@jfceurope.com **Web:** www.jfceurope.com

## **Polish Sales Office**

JFC Polska Trojany-Karpin 1A 05-520 Drabowka Poland **Tel:** (+) 48 (0) 297 578377 **Fax:** (+) 48 (0) 297 578201 **Email:** info@jfceurope.com **Web:** www.jfcpolska.com



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# APPENDIX D

# **OIL INTERCEPTOR DETAILS**

# SEPARATORS

A RANGE OF FUEL/OIL SEPARATORS FOR PEACE OF MIND





# Separators

# A RANGE OF FUEL/OIL SEPARATORS FOR PEACE OF MIND

Surface water drains normally discharge to a watercourse or indirectly into underground waters (groundwater) via a soakaway. Contamination of surface water by oil, chemicals or suspended solids can cause these discharges to have a serious impact on the receiving water.

The Environment Regulators, Environment Agency, England and Wales, SEPA, Scottish Environmental Protection Agency in Scotland and Department of Environment & Heritage in Northern Ireland, have published guidance on surface water disposal, which offers a range of means of dealing with pollution both at source and at the point of discharge from site (so called 'end of pipe' treatment). These techniques are known as 'Sustainable Drainage Systems' (SuDS).

Where run-off is draining from relatively low risk areas such as car-parks and non-operational areas, a source control approach, such as permeable surfaces or infiltration trenches, may offer a suitable means of treatment, removing the need for a separator.

Oil separators are installed on surface water drainage systems to protect receiving waters from pollution by oil, which may be present due to minor leaks from vehicles and plant, from accidental spillage.

Effluent from industrial processes and vehicle washing should normally be discharged to the foul sewer (subject to the approval of the sewerage undertaker) for further treatment at a municipal treatment works.

# SEPARATOR STANDARDS AND TYPES

A British (and European) standard (EN 858-1 and 858-2) for the design and use of prefabricated oil separators has been adopted. New prefabricated separators should comply with the standard.

### **SEPARATOR CLASSES**

The standard refers to two 'classes' of separator, based on performance under standard test conditions.

### CLASS I

Designed to achieve a concentration of less than 5mg/l of oil under standard test conditions, should be used when the separator is required to remove very small oil droplets.

# CLASS II

Designed to achieve a concentration of less than 100mg/l oil under standard test conditions and are suitable for dealing with discharges where a lower quality requirement applies (for example where the effluent passes to foul sewer).

Both classes can be produced as full retention separators. The oil concentration limits of 5 mg/l and 100 mg/l are only applicable under standard test conditions. It should not be expected that separators will comply with these limits when operating under field conditions.

# **FULL RETENTION SEPARATORS**

Full retention separators treat the full flow that can be delivered by the drainage system, which is normally equivalent to the flow generated by a rainfall intensity of 65mm/hr.

On large sites, some short term flooding may be an acceptable means of limiting the flow rate and hence the size of full retention systems. Get in touch for a FREE professional site visit and a representative will contact you within 5 working days to arrange a visit.

helpingyou@klargester.com to make the right decision or call 028 302 66799

# **BYPASS SEPARATORS**

Bypass separators fully treat all flows generated by rainfall rates of up to 6.5mm/hr. This covers over 99% of all rainfall events. Flows above this rate are allowed to bypass the separator. These separators are used when it is considered an acceptable risk not to provide full treatment for high flows, for example where the risk of a large spillage and heavy rainfall occurring at the same time is small.

# FORECOURT SEPARATORS

Forecourt separators are full retention separators specified to retain on site the maximum spillage likely to occur on a petrol filling station. They are required for both safety and environmental reasons and will treat spillages occurring during vehicle refuelling and road tanker delivery. The size of the separator is increased in order to retain the possible loss of the contents of one compartment of a road tanker, which may be up to 7,600 litres.

# SELECTING THE RIGHT SEPARATOR

The chart on the following page gives guidance to aid selection of the appropriate type of fuel/oil separator for use in surface water drainage systems which discharge into rivers and soakaways.

For further detailed information, please consult the Environment Agency Pollution Prevention Guideline 03 (PPG 3) 'Use and design of oil separators in surface water drainage systems' available from their website.

Kingspan Klargester has a specialist team who provide technical assistance in selecting the appropriate separator for your application.



5 Drainage from higher risk areas such as vehicle maintenance yards and goods vehicle parking areas should be connected to foul sewer in preference to surface water.

² You must seek prior permission from the relevant environmental body before you decide which separator to install.

In this case, if it is considered that there is a low risk of pollution a source control SuDS scheme may be appropriate. 3

⁴ In certain circumstances, the sewer provider may require a Class 1 separator for discharges to sewer to prevent explosive atmospheres from being generated.

⁶ In certain circumstances, a separator may be one of the devices used in the SuDS scheme. Ask us for advice.

# **Bypass** NSB RANGE

# **APPLICATION**

Bypass separators are used when it is considered an acceptable risk not to provide full treatment, for very high flows, and are used, for example, where the risk of a large spillage and heavy rainfall occurring at the same time is small, e.g.

- Surface car parks.
- Roadways.
- Lightly contaminated commercial areas.

# PERFORMANCE

Klargester were one of the first UK manufacturers to have separators tested to EN 858-1. Klargester have now added the NSB bypass range to their portfolio of certified and tested models. The NSB number denotes the maximum flow at which the separator treats liquids. The British Standards Institute (BSI) tested the required range of Kingspan Klargester Bypass separators and certified their performance in relation to their flow and process performance assessing the effluent gualities to the requirements of EN 858-1. Klargester bypass separator designs follow the parameters determined during the testing of the required range of bypass separators.

Each bypass separator design includes the necessary volume requirements for:

- Oil separation capacity. Oil storage volume. .
- Silt storage capacity. **.**

The unit is designed to treat 10% of peak flow. The calculated drainage areas served by each separator are indicated according to the formula given by PPG3 NSB = 0.0018A(m2). Flows generated by higher rainfall rates will pass through part of the separator and bypass the main separation chamber.

.

Coalescer.

Class I separators are designed to achieve a concentration of 5mg/litre of oil under standard test conditions.

## **FEATURES**

- Light and easy to install.
- Inclusive of silt storage volume.
- Fitted inlet/outlet connectors.
- н. Vent points within necks.
- Oil alarm system available (required by EN 858-1 and PPG3).

ire less

- н. Extension access shafts for deep inverts.
- Maintenance from ground level.
- GRP or rotomoulded construction (subject to model). н.

To specify a nominal size bypass separator, the following information is needed:-

- The calculated flow rate for the drainage area served. Our designs are based on the assumption that any interconnecting pipework fitted elsewhere on site does not impede flow into or out of the separator and that the flow is not pumped.
- The drain invert inlet depth.
- Pipework type, size and orientation.

#### STANDARD DRAINAGE UNIT FLOW PEAK FLOW STORAGE UNIT UNIT DIA. ACCESS BASE TO BASE TO STANDARD MIN. INLET NOMINAL CAPACITY (litres) INLET INVERT FALL ACROSS (l/s) RATE (I/s) AREA (m²) LENGTH (mm) (mm) SHAFT OUTLET INVERT PIPEWORK OIL SIZE DIA. (mm) INVERT DIA SILT (mm) (mm) (mm) NSBP003 NSBP004 NSBP006 NSBE010 NSBF015 NSBE020 NSBE025 NSBE030 NSBE040 NSBE050 NSBF075 NSBF100 NSBE125

SIZES AND SPECIFICATIONS

# Full Retention NSF RANGE

# **APPLICATION**

Full retention separators are used in high risk spillage areas such as:

- Fuel distribution depots.
- Vehicle workshops.
- Scrap Yards

### PERFORMANCE

Kingspan Klargester were the first UK manufacturer to have the required range (3-30 l/sec) certified to EN 858-1 in the UK. The NSF number denotes the flow at which the separator operates.

The British Standards Institute (BSI) have witnessed the performance tests of the required range of separators and have certified their performance, in relation to their flow and process performance to ensure that they met the effluent quality requirements of EN 858-1. Larger separator designs have been determined using the formulas extrapolated from the test range.

Each full retention separator design includes the necessary volume requirements for:

- Oil storage volume.
- Oil separation capacity. Silt storage capacity.
- Coalescer (Class I units only).
- Automatic closure device.

Klargester full retention separators treat the whole of the specified flow.

#### **FEATURES**

- Light and easy to install.
- Class I and Class II designs.
- 3-30 l/sec range independently tested and performance sampled, certified by the BSI.
- Inclusive of silt storage volume.
- Fitted inlet/outlet connectors. .

- Oil alarm system available.
- Vent points within necks.
- Extension access shafts for deep inverts.
- Maintenance from ground level.
- GRP or rotomoulded construction (subject to model).

To specify a nominal size full retention separator, the following information is needed:-

■ The calculated flow rate for the drainage area served. Our designs are based on the assumption that any interconnecting pipework fitted elsewhere on site does not impede flow into or out of the separator and that the influent is not pumped.

Kingspan Klargester

Advanced omoulded construction on selected models

Compact and robust

equire less backfill

, lightweight and

- The required discharge standard. This will decide whether a Class I or Class II unit is required.
- The drain invert inlet depth.
- Pipework type, size and orientation.

#### SIZES AND SPECIFICATIONS

UNIT Nominal	FLOW (I/s)	DRAINAGE AREA (m²) PPG-3 (0.018)	STORAGI (li	E CAPACITY tres)	UNIT LENGTH (mm)	UNIT DIA. (mm)	BASE TO INLET INVERT	BASE TO OUTLET	MIN. INLET INLET (mm)	STANDARD PIPEWORK
SIZE			SILT	OIL			(mm)	INVERT		DIA. (mm)
NSFP003	3	170	300	30	1700	1350	1420	1345	500	160
NSFP006	6	335	600	60	1700	1350	1420	1345	500	160
NSFA010	10	555	1000	100	2610	1225	1050	1000	500	200
NSFA015	15	835	1500	150	3910	1225	1050	1000	500	200
NSFA020	20	1115	2000	200	3200	2010	1810	1760	1000	315
NSFA030	30	1670	3000	300	3915	2010	1810	1760	1000	315
NSFA040	40	2225	4000	400	4640	2010	1810	1760	1000	315
NSFA050	50	2780	5000	500	5425	2010	1810	1760	1000	315
NSFA065	65	3610	6500	650	6850	2010	1810	1760	1000	315
NSFA080	80	4445	8000	800	5744	2820	2500	2450	1000	300
NSFA100	100	5560	10000	1000	6200	2820	2500	2450	1000	400
NSFA125	125	6945	12500	1250	7365	2820	2500	2450	1000	450
NSFA150	150	8335	15000	1500	8675	2820	2550	2450	1000	525
NSFA175	175	9725	17500	1750	9975	2820	2550	2450	1000	525
NSFA200	200	11110	20000	2000	11280	2820	2550	2450	1000	600

Rotomoulded chamber construction GRP chamber construction

# Washdown & Silt

# APPLICATION

This unit can be used in areas such as car wash and other cleaning facilities that discharge directly into a foul drain, which feeds to a municipal treatment facility.

If emulsifiers are present the discharge must not be allowed to enter an NS Class I or Class II unit.

- Car wash.
- Tool hire depots.
- Truck cleansing.
- Construction compounds cleansing points.

### PERFORMANCE

Such wash down facilities must not be allowed to discharge directly into surface water but must be directed to a foul connection leading to a municipal treatment works as they utilise emulsifiers, soaps and detergents, which can dissolve and disperse the oils.



- Light and easy to install.
- Inclusive of silt storage volume.
- Fitted inlet/outlet connectors.
- Vent points within necks.
- Extension access shafts for deep inverts.
- Maintenance from ground level.

#### SIZES AND SPECIFICATIONS

REF.	TOTAL CAPACITY (litres)	MAX. REC. Silt	MAX. FLOW RATE (I/s)	LENGTH (mm)	DIAMETER (mm)	ACCESS SHAFT DIA. (mm)	BASE TO INLET INVERT (mm)	BASE TO OUTLET INVERT (mm)	STANDARD FALL ACROSS UNIT (mm)	MIN. INLET INVERT (mm)	STANDARD PIPEWORK DIA. (mm)	APPROX EMPTY (kg)
W1/010	1000	500	3	1123	1225	460	1150	1100	50	500	160	60
W1/020	2000	1000	5	2074	1225	460	1150	1100	50	500	160	120
W1/030	3000	1500	8	2952	1225	460	1150	1100	50	500	160	150
W1/040	4000	2000	11	3898	1225	460	1150	1100	50	500	160	180
W1/060	6000	3000	16	4530	1440	600	1360	1310	50	500	160	320
W1/080	8000	4000	22	3200	2020	600	2005	1955	50	500	160	585
W1/100	10000	5000	27	3915	2020	600	2005	1955	50	500	160	680
W1/120	12000	6000	33	4640	2020	600	2005	1955	50	500	160	770
W1/150	15000	7500	41	5435	2075	600	1940	1890	50	500	160	965
W1/190	19000	9500	52	6865	2075	600	1940	1890	50	500	160	1200

# Car Wash Silt Trap

# APPLICATION

Car Wash silt trap is designed for use before a separator in car wash applications to ensure effective silt removal.

# **FEATURES**

- FACTA Class B covers.
- Light and easy to install.
- Maintenance from ground level.



# Forecourt

# APPLICATION

The forecourt separator is designed for installation in petrol filling station forecourts and similar applications. The function of the separator is to intercept hydrocarbon pollutants such as petroleum and oil and prevent their entry to the drainage system, thus protecting the environment against hydrocarbon contaminated surface water run-off and gross spillage.

# PERFORMANCE

Operation ensures that the flow cannot exit the unit without first passing through the coalescer assembly.

In normal operation, the forecourt separator has sufficient capacity to provide storage for separated pollutants within the main chamber, but is also able to contain up to 7,600 litres of pollutant arising from the spillage of a fuel delivery tanker compartment on the petrol forecourt. The separator has been designed to ensure that oil cannot exit the separator in the event of a major spillage, subsequently the separator should be emptied immediately.

# **FEATURES**

- Light and easy to install.
- Inclusive of silt storage volume.
- Fitted inlet/outlet connectors.
- Vent points within necks.
- Extension access shafts for deep inverts.
- Maintenance from ground level.

#### SIZES AND SPECIFICATIONS

- Class I and Class II design.
- Oil storage volume.
- Coalescer (Class I unit only).
- Automatic closure device.
- Oil alarm system available.

# **INSTALLATION**

The unit should be installed on a suitable concrete base slab and surrounded with concrete or pea gravel backfill. See sales drawing for installation.

Kingspan Klargester

If the separator is to be installed within a trafficked area, then a suitable cover slab must be designed to ensure that loads are not transmitted to the unit.

The separator should be installed and vented in accordance with Health and Safety Guidance Note HS(G)41 for filling stations, subject to Local Authority requirements.

ENVIROCEPTOR CLASS	TOTAL CAP. (litres)	DRAINAGE AREA (m²)	MAX. FLOW RATE (I/s)	LENGTH (mm)	DIAMETER (mm)	ACCESS SHAFT DIA. (mm)	BASE TO INLET INVERT (mm)	BASE TO OUTLET INVERT (mm)	STD. FALL Across Unit (mm)	MIN. INLET INVERT (mm)	STD. PIPEWORK (mm)	EMPTY WEIGHT (kg)
I	10000	555	10	3963	1920	600	2110	2060	50	400	160	500
Ш	10000	555	10	3963	1920	600	2110	2060	50	400	160	500
I	10000	1110	20	3963	1920	600	2110	2060	50	400	200	500
II	10000	1110	20	3963	1920	600	2110	2060	50	400	200	500

# Alarm Systems

British European Standard EN 858-1 and Environment Agency Pollution Prevention Guideline PPG3 requires that all separators are to be fitted with an oil level alarm system and that it should be installed and calibrated by a suitably qualified technician so that it will respond to an alarm condition when the separator requires emptying.

- Easily fitted to existing tanks.
- Excellent operational range.
- Visual and audible alarm.
- Additional telemetry option.



# **PROFESSIONAL INSTALLERS**

Kingspan Klargester Accredited Installers Experience shows that correct installation is a prerequisite for the long-lasting and successful operation of any wastewater treatment product. This is why using an installer with the experience and expertise



to install your product is highly recommended.

#### Services include :

- Site survey to establish ground conditions and soil types
- Advice on system design and product selection
- Assistance on gaining environmental consents and building approvals
- Tank and drainage system installation
- Connection to discharge point and electrical networks
- Waste emptying and disposal

Discover more about the Accredited Installers and locate your local expert online.

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# COMMERCIAL WASTEWATER SOLUTIONS

- BIODISC® & ENVIROSAFE HIGH PERFORMANCE SEWAGE TREATMENT SYSTEMS
- PACKAGE PUMP STATIONS
- PUMPSTOR24 PUMPING SYSTEMS
- OIL/WATER SEPARATORS
- BELOW GROUND STORAGE TANKS
- GREASE & SILT TRAPS

# **CARE & MAINTENANCE**

Kingspan Environmental Services Who better to look after your treatment plant than the people who designed and built it?



Kingspan Environmental have a dedicated service division providing maintenance for wastewater products.

Factory trained engineers are available for site visits as part of a planned maintenance contract or on a one-off call out basis.

To find out more about protecting your investment and ensuring peace of mind, call us on:

# 0844 846 0500

or visit us online: www.kingspanenvservice.com





#### **RAINWATER SOLUTIONS**

- BELOW GROUND RAINWATER HARVESTING SYSTEMS
- ABOVE GROUND RAINWATER HARVESTING SYSTEMS

#### Klargester

UK: College Road North, Aston Clinton, Aylesbury, Buckinghamshire HP22 5EW Tel: +44 (0) 1296 633000 Fax: +44 (0) 1296 633001 Scottish Office: Tel: +44 (0) 1355 248484 email: klargester@kingspan.com

 Ireland:
 Unit 1a, Derryboy Road, Carnbane Business Park, Newry, Co. Down BT35 6QH

 NI Tel: +44 (0) 28 302 66799
 Fax: +44 (0) 28 302 60046
 ROI Tel: 048 302 66799
 Fax: 048 302 60046

 email:
 klargesterinfo@kingspan.com
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Certificate No. OHS 575489









Environmental

Part of

Certificate No. FM 575486

In keeping with Company policy of continuing research and development and in order to offer our clients the most advanced products, Kingspan Environmental reserves the right to alter specifications and drawings without prior notice.

# APPENDIX E

# STORMTECH CALCULATIONS

Microstrain Ltd		Page 1
Unit B3, Metropoint Business	Newtownholmes Rd	
Swords, Co. Dublin	100YRP + 20%	
Ireland		Micro
Date 2/14/2024 11:25 PM	Designed by Stormtech SC740	
File R24-0129 - NEWTOWNHOLMES	Checked by	Diamage
Innovyze	Source Control 2019.1	

### Summary of Results for 100 year Return Period (+20%)

	Stor	rm	Max	Max	Max	Max	Status
	Ever	nt	Level	Depth	Infiltration	Volume	
			(m)	(m)	(1/s)	(m³)	
15	min	Winter	0 490	0 490	5 6	335 3	ОК
30	min	Winter	0.450	0.450	5.0	151 2	0 K
50	min	Winter	0.004	0.004	5.0	4J4.2	O K
100		wincer	1 004	1 004	5.9	373.0	A U
120	mın	Winter	1.024	1.024	6.0	/00.3	ΟK
180	min	Winter	1.134	1.134	6.1	775.7	ОК
240	min	Winter	1.211	1.211	6.1	828.6	ОК
360	min	Winter	1.316	1.316	6.2	899.9	ΟK
480	min	Winter	1.383	1.383	6.2	945.8	ΟK
600	min	Winter	1.428	1.428	6.2	976.7	ΟK
720	min	Winter	1.459	1.459	6.3	997.9	ΟK
960	min	Winter	1.493	1.493	6.3	1021.4	ΟK
1440	min	Winter	1.500	1.500	6.3	1025.8	ОК
2160	min	Winter	1.452	1.452	6.3	993.1	ΟK
2880	min	Winter	1.403	1.403	6.2	959.5	ΟK
4320	min	Winter	1.285	1.285	6.2	879.2	ΟK
5760	min	Winter	1.155	1.155	6.1	790.3	ΟK
7200	min	Winter	1.024	1.024	6.0	700.6	ΟK
8640	min	Winter	0.897	0.897	5.9	613.5	ΟK

Half Drain Time : 1419 minutes.

	Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Time-Peak (mins)
15	min Winter	93.730	0.0	2.6
30	min Winter	63.708	0.0	41
60	min Winter	40.649	0.0	70
120	min Winter	25.298	0.0	128
180	min Winter	19.019	0.0	186
240	min Winter	15.498	0.0	244
360	min Winter	11.587	0.0	360
480	min Winter	9.415	0.0	476
600	min Winter	8.010	0.0	592
720	min Winter	7.017	0.0	706
960	min Winter	5.693	0.0	932
1440	min Winter	4.238	0.0	1364
2160	min Winter	3.154	0.0	1720
2880	min Winter	2.556	0.0	2188
4320	min Winter	1.897	0.0	3112
5760	min Winter	1.535	0.0	3984
7200	min Winter	1.301	0.0	4832
8640	min Winter	1.137	0.0	5704
	©1982-	-2019 I	nnovyze	

Microstrain Ltd					Page 2
Unit B3, Metropoint Business	Newtownh	olmes Rd			
Swords, Co. Dublin	100YRP +	20%			
Ireland					Micco
Date 2/14/2024 11:25 PM	Designed	by Storr	ntech S	2740	
File R24-0129 - NEWTOWNHOLMES	Checked	by			Digitigh
Innovyze	Source C	ontrol 20	019.1		
<u>Summary of Results f</u>	or 100 ye	ar Retur	n Perio	d (+20%)	
				<b>.</b>	
Storm Max Event Leve	K Max	Max	Max Volume	Status	
(m)	(m)	(1/s)	(m ³ )		
10080 min Winter 0.77	76 0.776	5.8	3 530.8	ОК	
Storm	Rain	Flooded T	'ime-Peak		
Event	(mm/hr)	Volume	(mins)		
		(m ³ )			
10080 min Wint	er 1.015	0.0	6464		
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Microstrain Ltd		Page 3
Unit B3, Metropoint Business	Newtownholmes Rd	
Swords, Co. Dublin	100YRP + 20%	
Ireland		Micro
Date 2/14/2024 11:25 PM	Designed by Stormtech SC740	Dcainago
File R24-0129 - NEWTOWNHOLMES	Checked by	Diamage
Innovyze	Source Control 2019.1	
<u>R</u>	ainfall Details	
Rainfall Model Return Period (years) Region Scotl M5-60 (mm) Ratio R Summer Storms	FSR Winter Storms Y 100 Cv (Summer) 0.7 and and Ireland Cv (Winter) 0.8 17.300 Shortest Storm (mins) 0.300 Longest Storm (mins) 100 No Climate Change % +	es 50 40 15 80 20
Ti	me Area Diagram	
То	tal Area (ha) 1.739	
Time (mins) Area T From: To: (ha) F	'ime (mins) Area Time (mins) Area rom: To: (ha) From: To: (ha)	
0 4 0.580	4 8 0.580 8 12 0.580	
©19	82-2019 Innovyze	

Microstrain Ltd		Page 4
Unit B3, Metropoint Business	Newtownholmes Rd	
Swords, Co. Dublin	100YRP + 20%	
Ireland		Micro
Date 2/14/2024 11:25 PM	Designed by Stormtech SC740	Desinado
File R24-0129 - NEWTOWNHOLMES	Checked by	Diamage
Innovyze	Source Control 2019.1	·

#### Model Details

Storage is Online Cover Level (m) 2.000

#### Cellular Storage Structure

Invert Level (m) 0.000 Safety Factor 2.0 Infiltration Coefficient Base (m/hr) 0.03370 Porosity 0.60 Infiltration Coefficient Side (m/hr) 0.03370

#### Depth (m) Area (m²) Inf. Area (m²) Depth (m) Area (m²) Inf. Area (m²)

0.000	1140.0	1140.0	1.700	0.0	1357.6
1.600	1140.0	1357.6			

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